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- G0.2

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- FM-01

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- FM-02

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- G1

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- NT30.0

DESIGN NOTES
- NT30.1

PLANS AND ELEVATIONS
- NT 30.2

SECTIONS AND DETAILS

ARCHITECT

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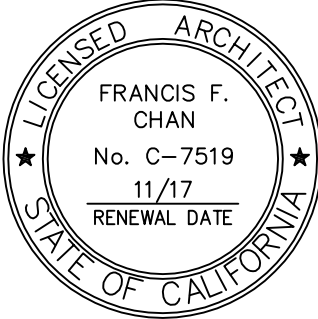
333 1ST STREET, SUITE C  
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303 POTRERO STREET, SUITE 7B  
SANTA CRUZ, CA 95060  
TEL: 800.725.0571

OWNER



CONSULTANT

PROFESSIONAL STAMP:



PROJECT:

LOUISE WILBOURN  
YARBROUGH  
HORTICULTURE &  
PLANT SCIENCE  
INSTITUTE  
4000 Suisun Valley Rd,  
Fairfield, CA 94534

REVISIONS

REF	DESCRIPTION	DATE
-	-	-

PROJECT CODE:SCCD-04

START DATE:-

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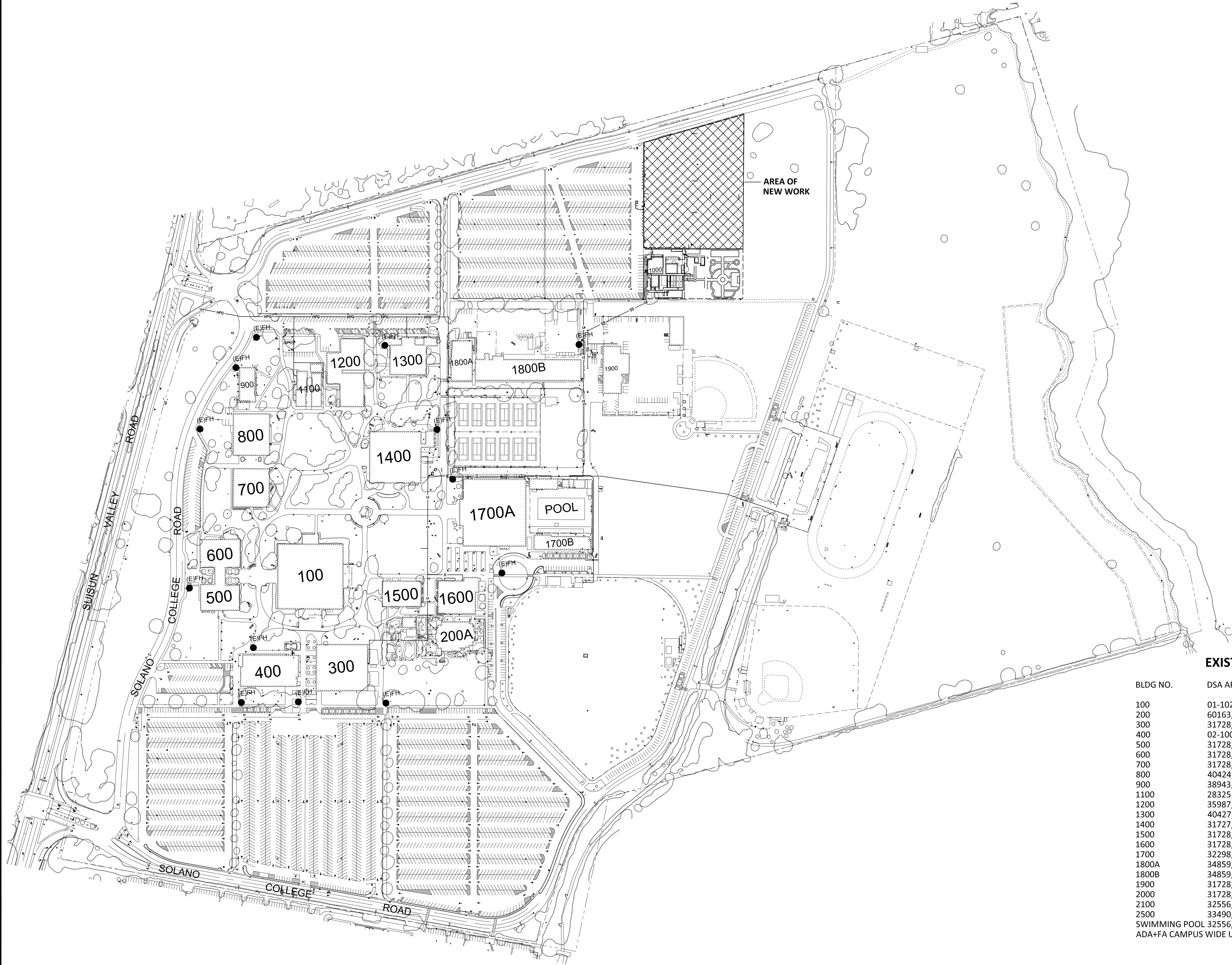
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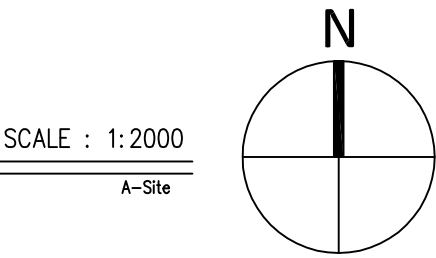
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EXISTING BUILDING SUMMARY

BLDG NO.	DSA APPLICATION NO.	AREA
100	01-102029, 31727, 43623	49,600 SF
200	60163, 43623	9,252 SF
300	31728, 43623, 31407	24,240 SF
400	02-100548	1,440 SF
500	31728, 43623, 31407	11,616 SF
600	31728, 43623 31407, 02-11343	13,056 SF
700	31728, 43623, 31407	16,864 SF
800	40424, 43623, 02-107608	17,856 SF
900	38943, 43623	2,447 SF
1100	28325	17,500 SF
1200	35987, 43623	25,251 SF
1300	40427, 43623	12,240 SF
1400	31727, 43623	30,976 SF
1500	31728, 43623, 31407	11,616 SF
1600	31728, 43623, 31407	14,336 SF
1700	32298, 43623, 02-103762	48,201 SF
1800A	34859, 43623	9,660 SF
1800B	34859, 43623, 02-106610	24,610 SF
1900	31728, 43623, 31407	10,730 SF
2000	31728, 31407	3,100 SF
2100	32556, 33013, 43623	3,500 SF
2500	33490, 33013, 43623	
SWIMMING POOL	32556, 43623	
ADA+FA CAMPUS WIDE UPGRADES	02-103473	

1 CAMPUS SITE MAP



ARCHITECT

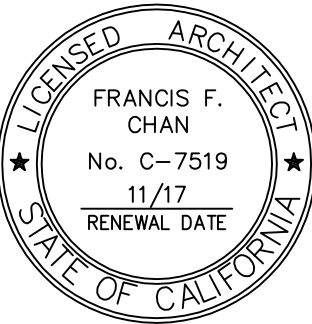
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CAMPUS  
SITE MAP

DSA APPROVAL STAMP:

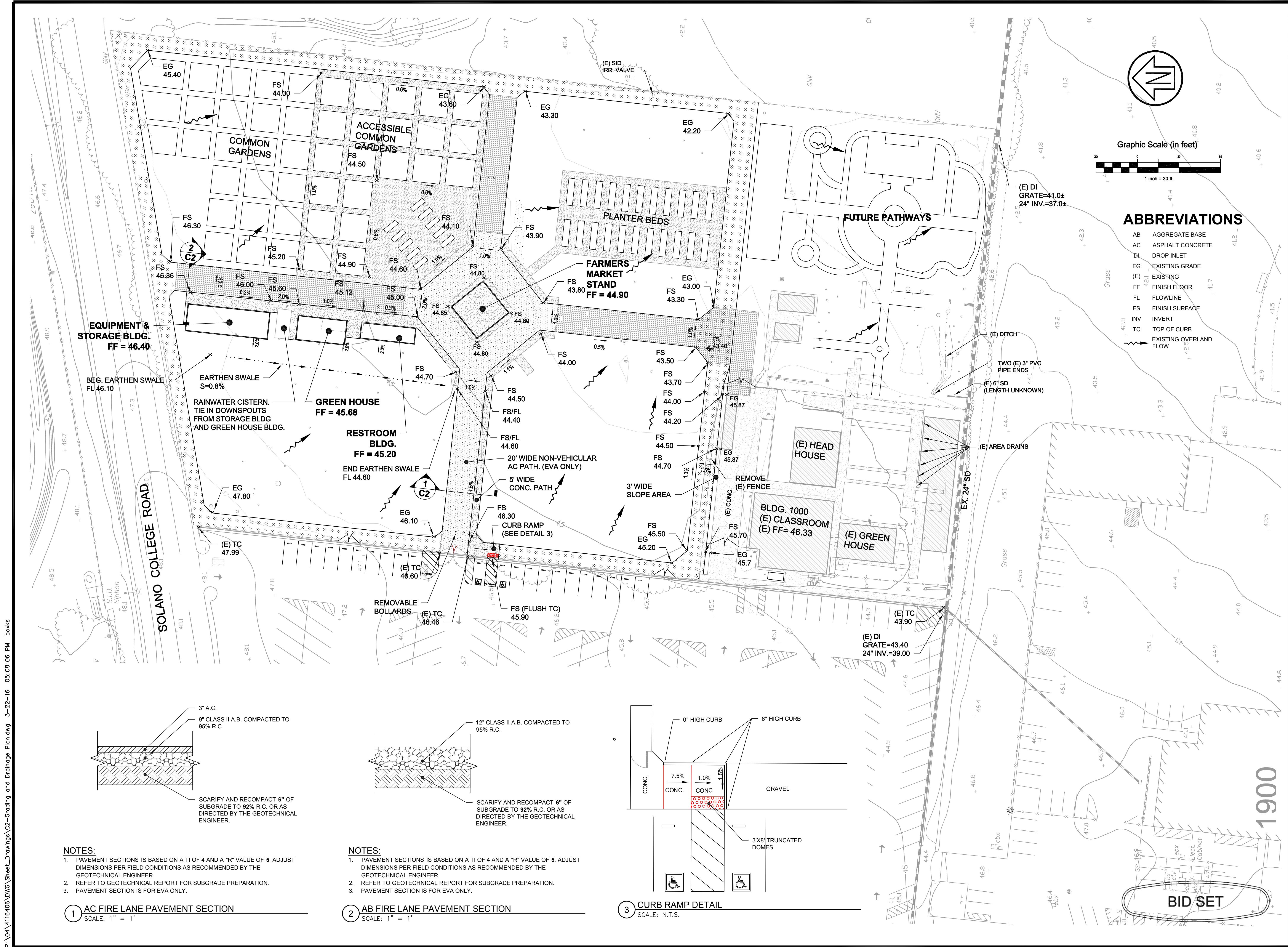
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CONSULTANT

**CSW ST2**

**CSW/Stuber-Stroeh**  
**Engineering Group, Inc.**

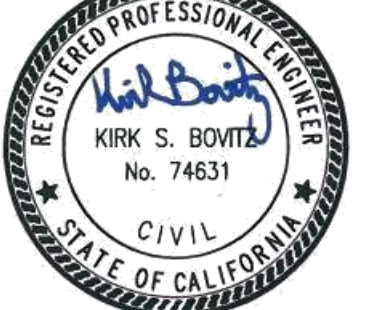
45 Leveroni Court Tel: 415.883.9850  
Novato, CA 94949 Fax: 415.883.9856

Civil & Structural Engineers  
Surveying & Mapping  
Environmental Planning  
Land Planning  
Construction Management

http://www.cswst2.com

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PROFESSIONAL



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**LOUISE WILBOURN  
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HORTICULTURE &  
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4000 Suisun Valley Rd,  
Fairfield, CA 94534

REVISIONS

REF	DESCRIPTION	DATE
D	VE REVISIONS	03/23/16
C	DSA BACKCHECK	03/07/16
B		
A	CLIENT SUBMITTAL	12/01/15

PROJECT CODE: SCCD-04

START DATE:

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**GRADING &  
DRAINAGE PLAN**

DSA APPROVAL STAMP:

SHEET NUMBER:

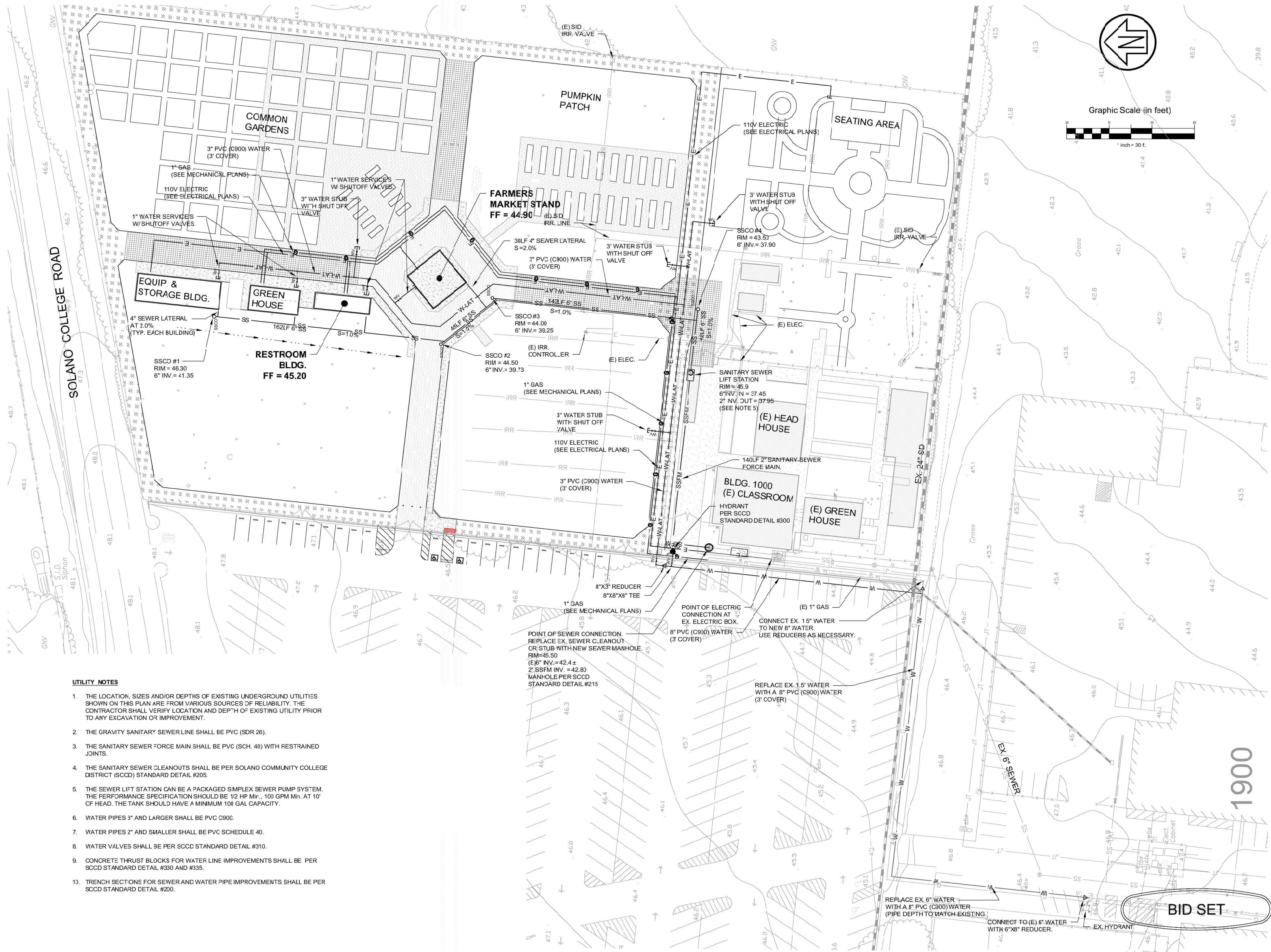
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SOLANO COMMUNITY COLLEGE DISTRICT





UTILITY NOTES

1. THE LOCATION, SIZES AND/OR DEPTHS OF EXISTING UNDERGROUND UTILITIES SHOWN ON THIS PLAN ARE FROM VARIOUS SOURCES OF RELIABILITY. THE CONTRACTOR SHALL VERIFY LOCATION AND DEPTH OF EXISTING UTILITY PRIOR TO ANY EXCAVATION OR IMPROVEMENT.
2. THE GRAVITY SANITARY SEWER LINE SHALL BE PVC (SDR 26).
3. THE SANITARY SEWER FORCE MAIN SHALL BE PVC (SCH. 40) WITH RESTRAINED JOINTS.
4. THE SANITARY SEWER CLEANOUTS SHALL BE PER SOLANO COMMUNITY COLLEGE DISTRICT (SCCD) STANDARD DETAIL #205.
5. THE SEWER LIFT STATION CAN BE A PACKAGED SIMPLEX SEWER PUMP SYSTEM. THE PERFORMANCE SPECIFICATION SHOULD BE 1/2 HP Min., 100 GPM Min. AT 10' OF HEAD. THE TANK SHOULD HAVE A MINIMUM 100 GAL CAPACITY.
6. WATER PIPES 3" AND LARGER SHALL BE PVC C900.
7. WATER PIPES 2" AND SMALLER SHALL BE PVC SCHEDULE 40.
8. WATER VALVES SHALL BE PER SCCD STANDARD DETAIL #310.
9. CONCRETE THRUST BLOCKS FOR WATER LINE IMPROVEMENTS SHALL BE PER SCCD STANDARD DETAIL #330 AND #335.
10. TRENCH SECTIONS FOR SEWER AND WATER PIPE IMPROVEMENTS SHALL BE PER SCCD STANDARD DETAIL #200.

ARCHITECT

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CONSULTANT

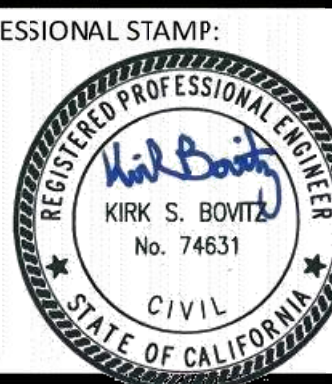
**CSW ST2**

**CSW/Stuber-Stroeh**  
**Engineering Group, Inc.**

45 Leveroni Court Tel: 415.853.9850  
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Civil & Structural Engineers  
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Environmental Planning  
Land Planning  
Construction Management

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A	CLIENT SUBMITTAL	12/01/15

PROJECT CODE: SCCD-04

START DATE:

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UTILITY PLAN

DSA APPROVAL STAMP:

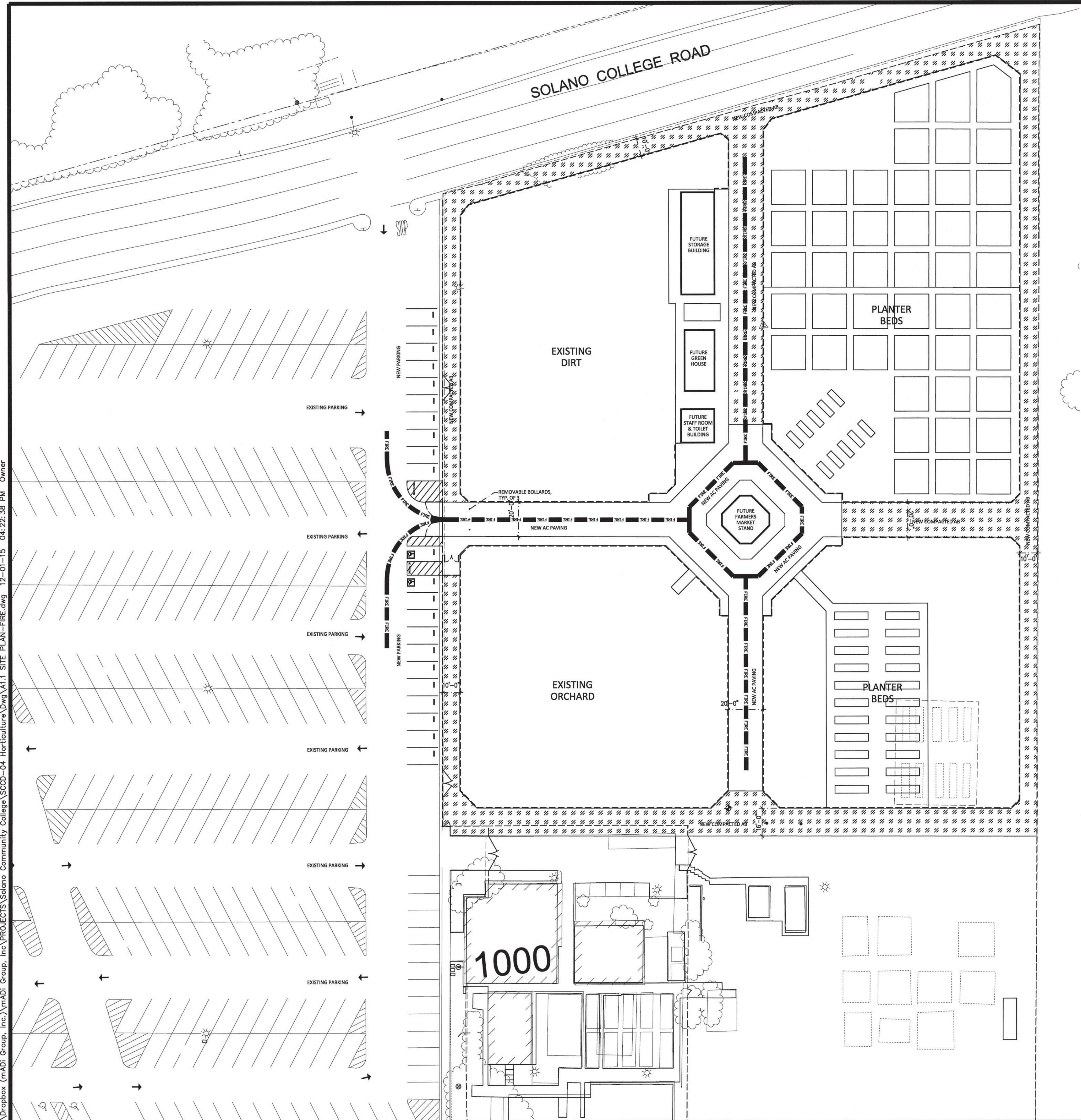
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1 SITE PLAN

SCALE: 1" = 30'-0"

DSA

810

## LOCAL FIRE AUTHORITY REVIEW

To facilitate the Division of the State Architect's (DSA) approval of the Fire/Life Safety portion of a project, DSA requires Local Fire Authority (LFA) review of certain elements as identified in this form. Use of this form is mandatory for projects that add square footage to a campus or if any item on this form is relevant to the project. For additional information, see DSA 810 Instructions and DSA Policy 00-01.

PROJECT INFORMATION	
School District/Owner:	
Project Name/School:	
Project Address:	
LOCAL FIRE AUTHORITY (LFA)	
LFA Agency Name: <u>Cosumnes Fire Protection District</u>	
LFA Reviewer Name: <u>Keith Martin</u>	Title: <u>Fire Chief</u>
Email: <u>keith.martin@cosumnes.net</u>	Telephone Number: <u>707-564-0468</u>
I have reviewed and responded to the applicable items for this project as listed below.	
Note: Only sign this form when it is imaged onto the site plan. A loose form is not acceptable to DSA.	
LFA Reviewer's Signature: <u>[Signature]</u> Date: <u>12/14/15</u>	
Review Key: "Y" = Complies with LFA requirements "N" = Not approved (complete Section 8) "NA" = Not applicable to the project "NR" = LFA elects not to review	
Description	Y N NA NR
1 Where an elevator does not meet medical emergency service cab size, per the California Building Code (CBC), use of stairways for emergency rescue and patient transport is acceptable.	
2 Access roads, fire lane markings, pavers and gate entrances are in accordance with Title 19, California Code of Regulations and the California Fire Code, Chapter 5.	X
3 Fire hydrant location and distribution complies with the California Fire Code (or see # 4).	X
4 Fire hydrant location and distribution complies with NFPA 1142, "Alternate Means." If "NR" is checked, DSA can only approve on-site water storage as an alternate. The signature of the school district official is required to acknowledge the use of alternate means.	X
Signature of School District Official: _____ Date: _____	
Print the School District Official's Name: _____	
5 The location(s) of the proposed post indicator valve and fire department connection meet the requirements of this jurisdiction.	X
6 The location(s) of the detector check valve assembly meet the requirements of this jurisdiction.	X
7 Is the project located in a hazard severity zone area? (CBC, Chapter 7A, Section 701A.) <input type="checkbox"/> Yes <input checked="" type="checkbox"/> No	
Check type if "Yes": <input type="checkbox"/> Moderate <input type="checkbox"/> High <input type="checkbox"/> Very High <input type="checkbox"/> WIFA (If one of these boxes is checked, the project design must meet the requirements of Chapter 7A.)	
COMMENTS (note deficiencies):	
8	

DSA 810 (rev 05-12-14) DIVISION OF THE STATE ARCHITECT DEPARTMENT OF GENERAL SERVICES Page 1 of 1 STATE OF CALIFORNIA

### FIRE TRUCK ACCESS

THE DESIGNATION OF THE FIRE LANE(S) SHALL BE INDICATED PER THE CALIFORNIA VEHICLE CODE SECTION 22500.1(3) BY OUTLINING OR PAINTING THE LANES IN RED, AND IN CONTRASTING COLOR, MARKING THE LANES WITH THE WORDS 'FIRE LANE', WHICH ARE CLEARLY VISIBLE FROM A VEHICLE. MARKED FIRE LANES SHALL BE A MINIMUM OF 20'-0" WIDE.

LEGEND: FIRE TRUCK ACCESS (FIRE LANE)  
 EXISTING FIRE HYDRANT

ARCHITECT

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SANTA CRUZ, CA 95060

TEL: 800.725.0571

OWNER



CONSULTANT

PROFESSIONAL STAMP:



PROJECT:

LOUISE WILBOURN  
YARBROUGH  
HORTICULTURE &  
PLANT SCIENCE  
INSTITUTE

4000 Suisun Valley Rd,  
Fairfield, CA 94534

REVISIONS

REF	DESCRIPTION	DATE

PROJECT CODE: SCCD-04

START DATE: -

DRAWN BY: -

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SHEET NAME:

SITE PLAN  
FIRE MARSHAL  
REVIEW

DSA APPROVAL STAMP:

SHEET NUMBER:

A1.1

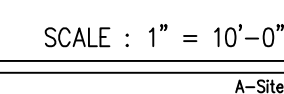
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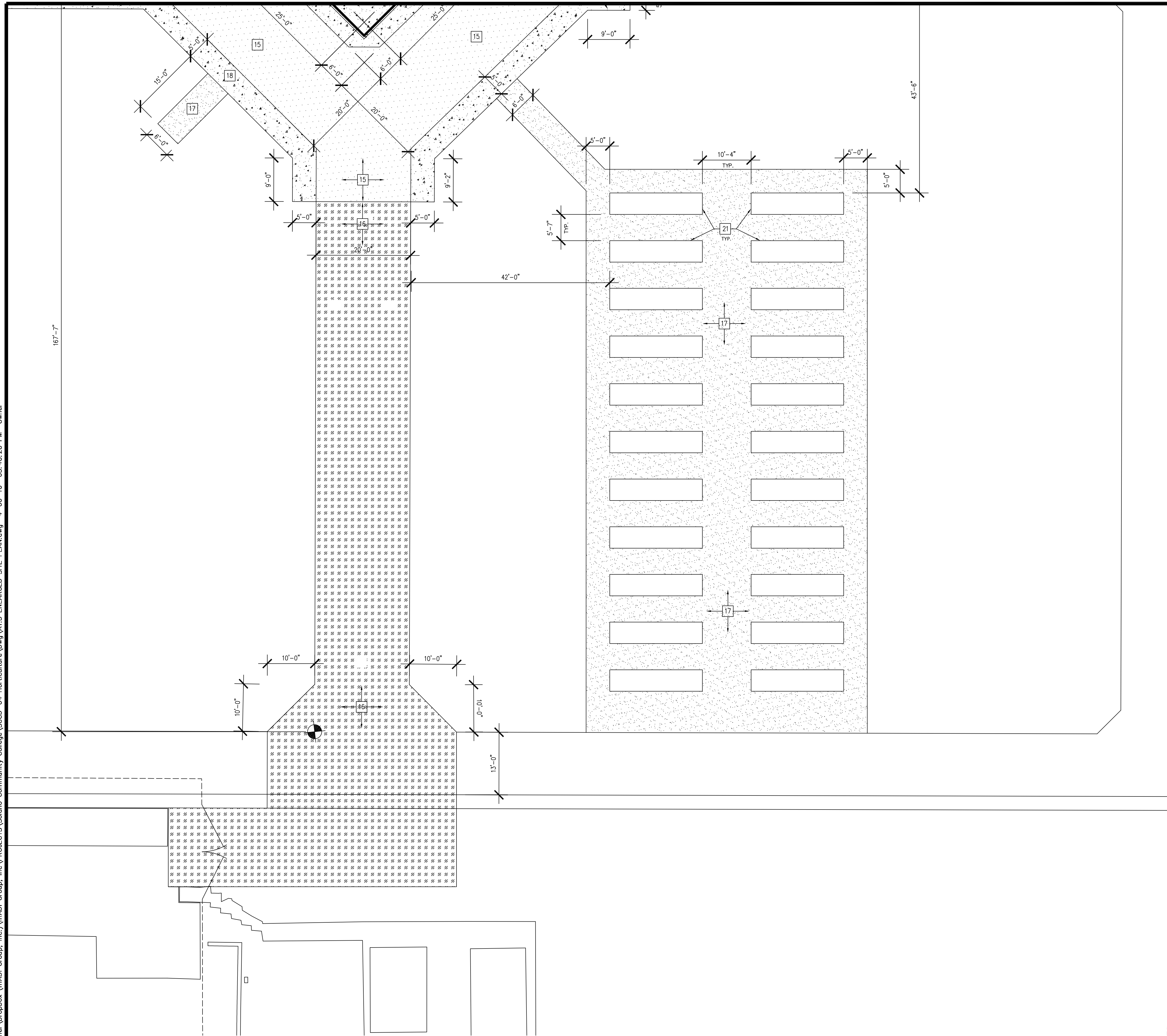
1	DEMOLISH EXISTING FENCE AND/OR GATE
2	EXISTING FENCE TO REMAIN
3	(N) 20'-0" WIDE X 6'-0" HIGH SWING GATE; SEE 6B/A1.9
4	(N) DETECTABLE WARNING STRIPS; SEE 7&8/A1.8
5	(N) 3'-0" WIDE X 6'-0" HIGH SWING GATE; SEE 6A/A1.9
6	(N) ACCESSIBLE PARKING; SEE 1/A1.8
7	(N) PARKING STRIPING
8	(N) WHEELSTOP; SEE 9/A1.8
9	(N) REMOVABLE BOLLARDS; SEE 4/A1.8
10	(N) SIGNAGE; SEE 10/A1.8
11	(N) SIGNAGE; SEE 11/A1.8
12	(N) SIGNAGE; SEE 12/A1.8
13	(N) 4'-0" HIGH CHAINLINK FENCE
14	(N) 6'-0" HIGH CHAINLINK FENCE
15	(N) AC PAVING
16	(N) AB PAVING
17	(N) DG PAVING. PROVIDE REDWOOD HEADER EDGING PER DETAIL 5/A1.8
18	(N) CONCRETE PAVING
19	(N) 20'X20' PLANTERS FLUSH WITH ADJACENT GRADE WITH REDWOOD HEADER EDGING PER DETAIL 5/A1.8
20	(N) 3'-6"X11'-0" PLANTERS
21	(N) 4'-6"X20'-0" PLANTERS
22	(N) 4'-0" HIGH X 5'-0" SLIDING GATE; SEE 1/A1.9. PROVIDE SIGN STATING "ENTRY CONTROLLED AND RESTRICTED BY SECURITY PERSONNEL" PER CIB 118-404.1 EXCEPTION 1.
23	(E) DIRT
24	(N) SIGNAGE; SEE 3/A1.8

## A1.3



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1 ENLARGED SITE PLAN

SCALE : 1" = 10'-0"

LEGEND

- |    |  |
|----|--|
| 1  | DEMOLISH EXISTING FENCE AND/OR GATE  |
| 2  | EXISTING FENCE TO REMAIN   |
| 3  | (N) 20'-0" WIDE X 6'-0" HIGH SWING GATE; SEE 6B/A1.9   |
| 4  | (N) DETECTABLE WARNING STRIPS; SEE 7&8/A1.8  |
| 5  | (N) 3'-0" WIDE X 6'-0" HIGH SWING GATE; SEE 6A/A1.9  |
| 6  | (N) ACCESSIBLE PARKING; SEE 1/A1.8   |
| 7  | (N) PARKING STRIPING   |
| 8  | (N) WHEELSTOP; SEE 9/A1.8  |
| 9  | (N) REMOVABLE BOLLARDS; SEE 4/A1.8   |
| 10 | (N) SIGNAGE; SEE 10/A1.8   |
| 11 | (N) SIGNAGE; SEE 11/A1.8   |
| 12 | (N) SIGNAGE; SEE 12/A1.8   |
| 13 | (N) 4'-0" HIGH CHAINLINK FENCE   |
| 14 | (N) 6'-0" HIGH CHAINLINK FENCE   |
| 15 | (N) AC PAVING  |
| 16 | (N) AB PAVING  |
| 17 | (N) DG PAVING. PROVIDE REDWOOD HEADER EDGING PER DETAIL 5/A1.8   |
| 18 | (N) CONCRETE PAVING  |
| 19 | (N) 20'X20' PLANTERS FLUSH WITH ADJACENT GRADE WITH REDWOOD HEADER EDGING PER DETAIL 5/A1.8  |
| 20 | (N) 3'-6"X11'-0" PLANTERS  |
| 21 | (N) 4'-6"X20'-0" PLANTERS  |
| 22 | (N) 4'-0" HIGH X 5'-0" SLIDING GATE; SEE 1/A1.9. PROVIDE SIGN STATING "ENTRY CONTROLLED AND RESTRICTED BY SECURITY PERSONNEL" PER CBC 118-404.1 EXCEPTION 1. |
| 23 | (E) DIRT   |
| 24 | (N) SIGNAGE; SEE 3/A1.8  |

ARCHITECT

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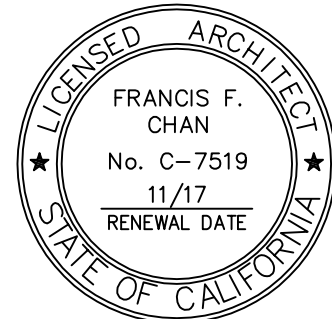
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YARBROUGH  
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PLANT SCIENCE  
INSTITUTE

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Fairfield, CA 94534

## REVISIONS

[illegible]

PROJECT CODE: SCCD-04

START DATE

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# ENLARGED SITE PLAN

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SHEET NUMBER:

# A1.5

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[illegible][illegible]

1	DEMOLISH EXISTING FENCE AND/OR GATE
2	EXISTING FENCE TO REMAIN
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23	(E) DIRT
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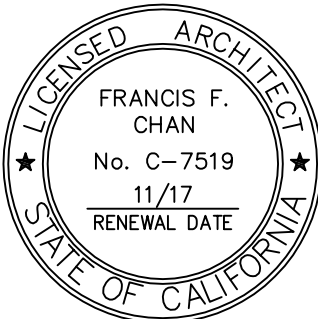
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# ENLARGED SITE PLAN

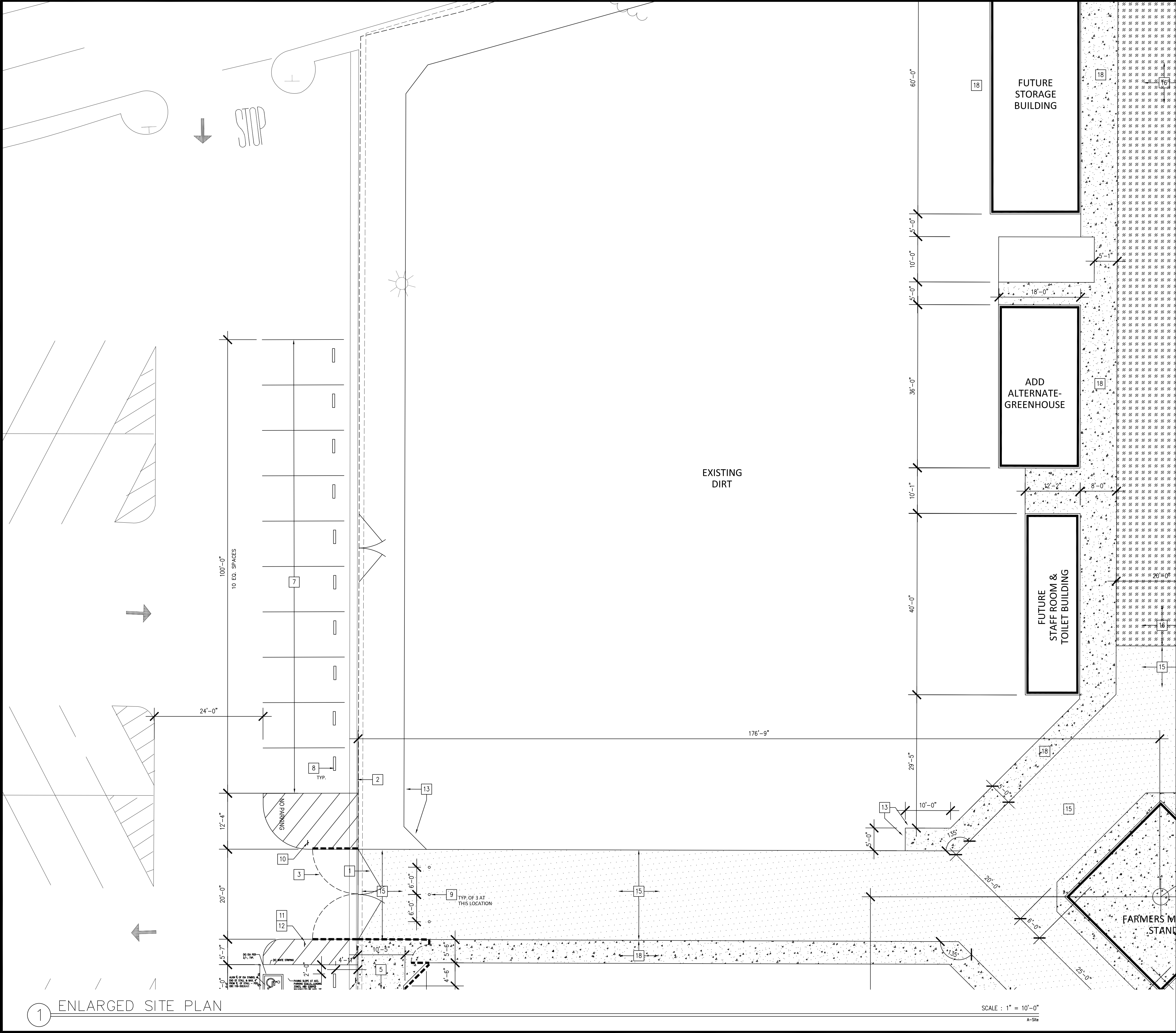
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SHEET NUMBER:

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**LEGEND**

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2	EXISTING FENCE TO REMAIN
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21	(N) 4'-6"X20'-0" PLANTERS
22	(N) 4'-0" HIGH X 5'-0" SLIDING GATE; SEE 1/A1.9. PROVIDE SIGN STATING "ENTRY CONTROLLED AND RESTRICTED BY SECURITY PERSONNEL" PER CBC 118-404.1 EXCEPTION 1.
23	(E) DIRT
24	(N) SIGNAGE; SEE 3/A1.8

ARCHITECT

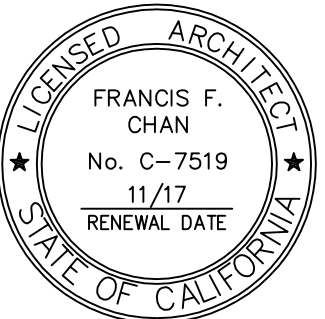


OWNER



CONSULTANT

PROFESSIONAL STAMP:



PROJECT:

LOUISE WILBOURN  
YARBROUGH  
HORTICULTURE &  
PLANT SCIENCE  
INSTITUTE

4000 Suisun Valley Rd  
Fairfield, CA 94534

[illegible]

PROJECT CODE: SCCD-04

START DATE

DRAWN BY:

CHECKED BY

SHEET NAME:

# ENLARGED SITE PLAN

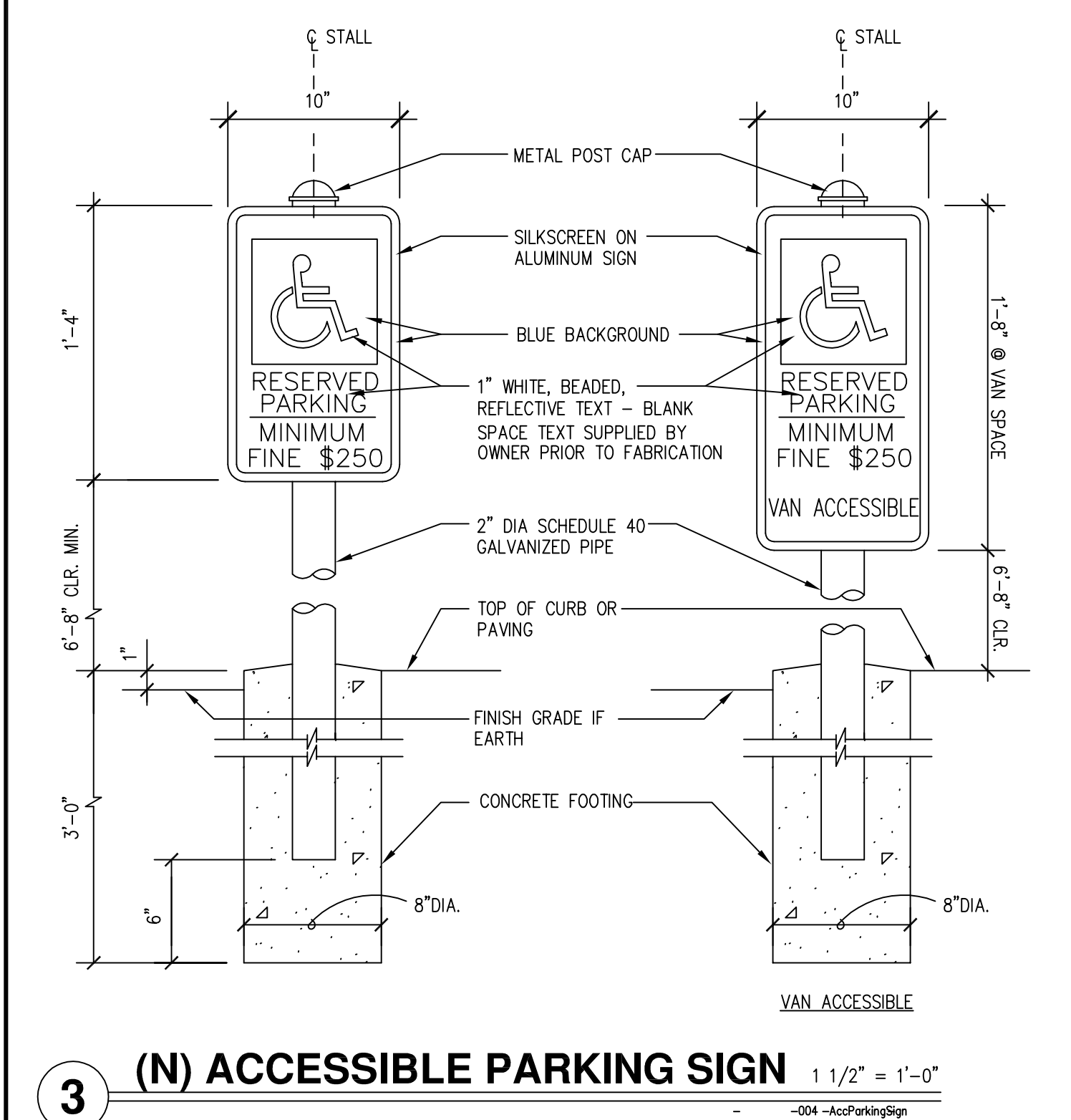
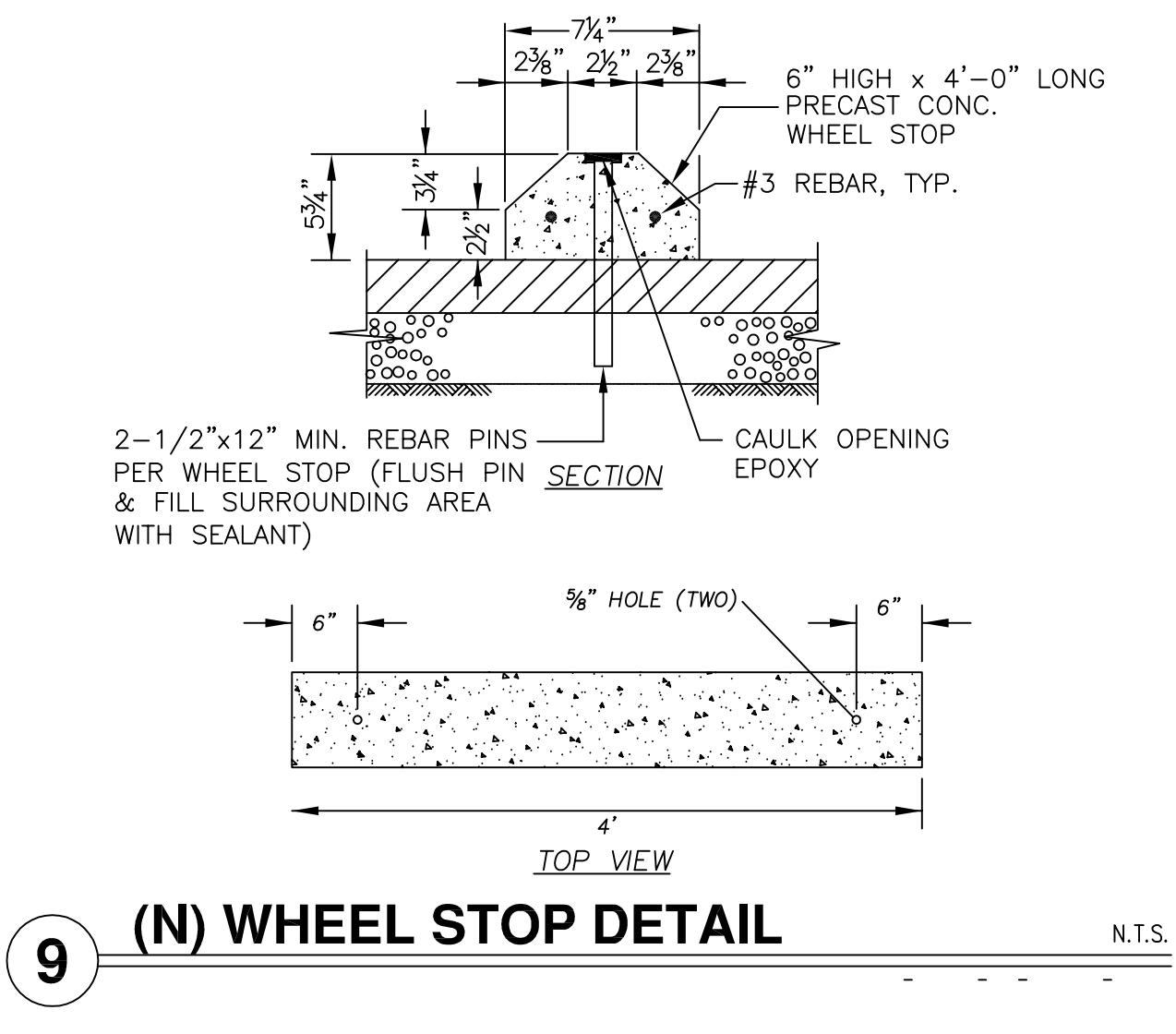
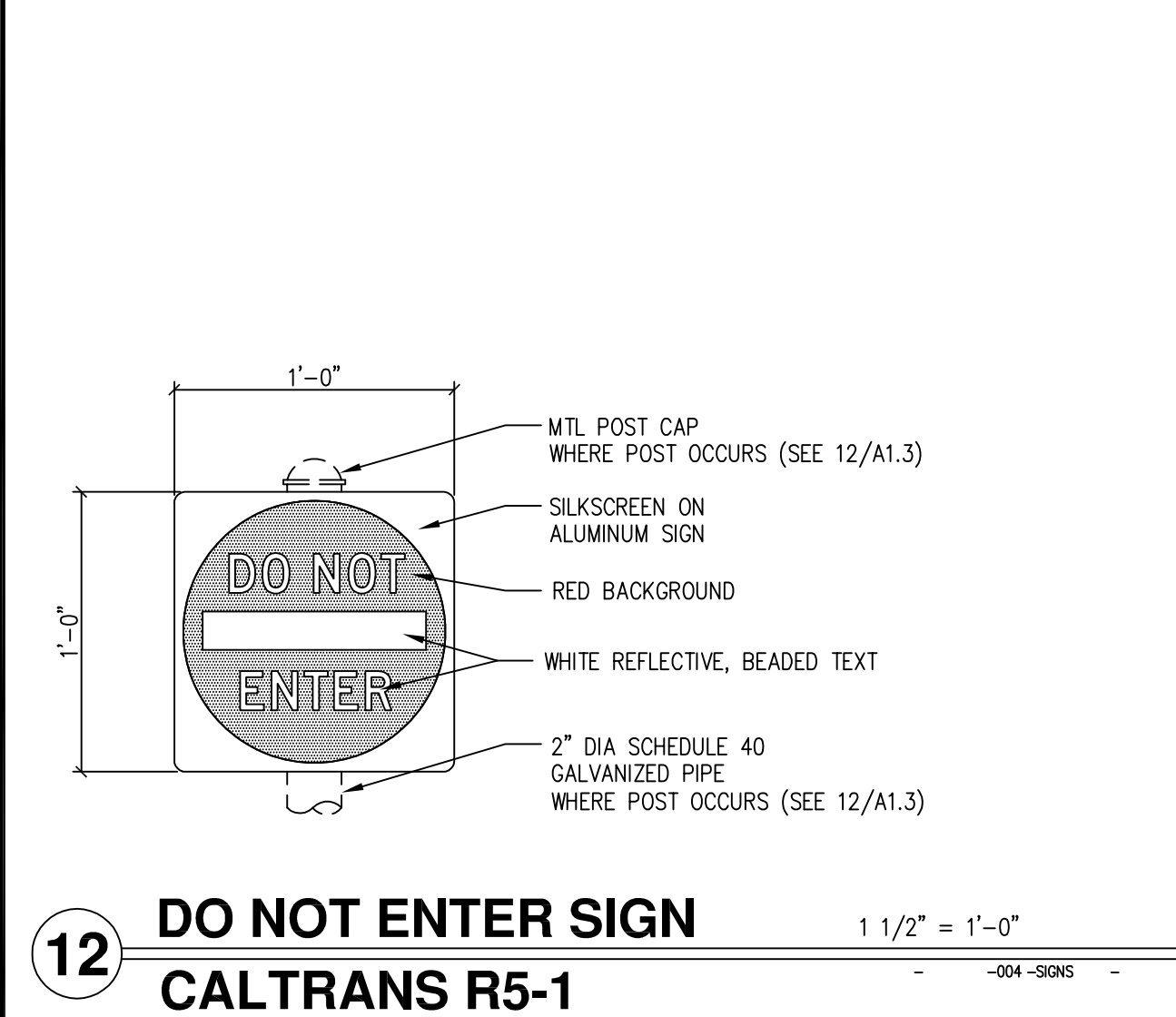
DSA APPROVAL STAMP

SHEET NUMBER:

## A1.7

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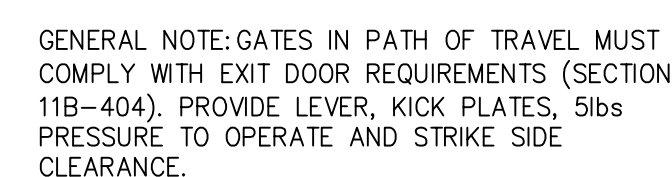
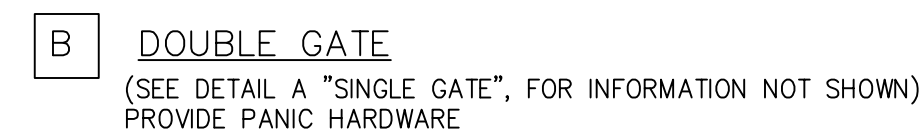








**A** SINGLE GATE  
PROVIDE PANIC HARDWARE

PROJECT CODE: SCCD-04

START DATE: \_\_\_\_\_

DRAWN BY: -

CHECKED BY: \_\_\_\_\_

SHEET NAME:

DSA APPROVAL STAMP

SHEET NUMBER:









LEGEND

- 1 STANDING SEAM METAL ROOFING SYSTEM, TYP. 24 GA. MIN. "COOL ROOF" FINISH.
- 2 GALV. SHEET METAL GUTTER, FINISH TO MATCH ROOFING PANELS, TYP. PROVIDE DOWNSPOUT TO GRADE AT EACH POST
- 3 SUPPORT POST, TYP. PER MFR. SQUARE HSS STEEL TUBE W/ CAST-IN-PLACE CONCRETE FOOTING. ALL SURFACES PAINTED.
- 4 ROOF STRUCTURE PER MFR, TYP. HSS STEEL TUBES THROUGHOUT, W/ WELDED OR CONCEALED BOLTED CONNECTIONS. ALL SURFACES PAINTED.
- 5 EAVE BEAM, HSS STEEL TUBE SIM. TO ROOF STRUCTURE. ALL SURFACES PAINTED
- 6 CEILING FAN, SUSPEND FROM HIGH POINT OF ROOF FRAMING. CONCEAL ELECTRICAL CONDUIT INSIDE STEEL FRAMING.
- 7 NOT USED

## GENERAL NOTES

1. THESE DRAWINGS ARE TO SHOW DESIGN INTENT ONLY. BUILDING MFR. TO PROVIDE FINAL DRAWINGS TO ARCHITECT, FOR DSA REVIEW AND APPROVAL.
2. UTILITY LINES WITHIN 5' OF BUILDING FOOTPRINT ARE TO BE IDENTIFIED BY BUILDING CONTRACTOR, IN COORDINATION WITH SITE CONTRACTOR.
3. FIRE ALARM SYSTEM WITHIN BUILDING TO BE INSTALLED BY SITE CONTRACTOR, IN COORDINATION WITH BUILDING MFR.
4. BUILDING TO COMPLY WITH ALL APPLICABLE PROVISIONS OF THE 2013 CALIFORNIA BUILDING CODE.
5. BUILDING FLOOR/FOUNDATION SYSTEM TO BE CAST-IN-PLACE CONCRETE FOOTINGS, BY BUILDING CONTRACTOR.
6. BUILDING PAD TO BE PROVIDED BY SITE CONTRACTOR PER BUILDING MFR'S REQUIREMENTS.
7. PAVEMENT UNDER AND ADJACENT TO BUILDING TO BE PROVIDED BY SITE CONTRACTOR.

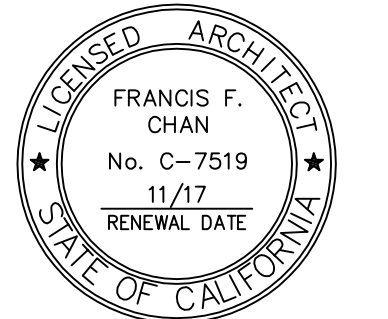
**MADI**  
ARCHITECTURE + PLANNING

333 1ST STREET, SUITE C  
SAN FRANCISCO, CA 94105  
303 POTRERO STREET, SUITE 7B  
SANTA CRUZ, CA 95060

TEL: 800.725.0571



CONSULTANT



LOUISE WILBOURN  
YARBROUGH  
HORTICULTURE &  
PLANT SCIENCE  
INSTITUTE

## REVISIONS

[illegible]

START DATE:

DRAWN BY: \_\_\_\_\_

CHECKED BY:

SHEET NAME:

## FARM STAND PLANS AND ELEVATIONS

DSA APPROVAL STAMP:

SHEET NUMBER:

**A2.5A**  
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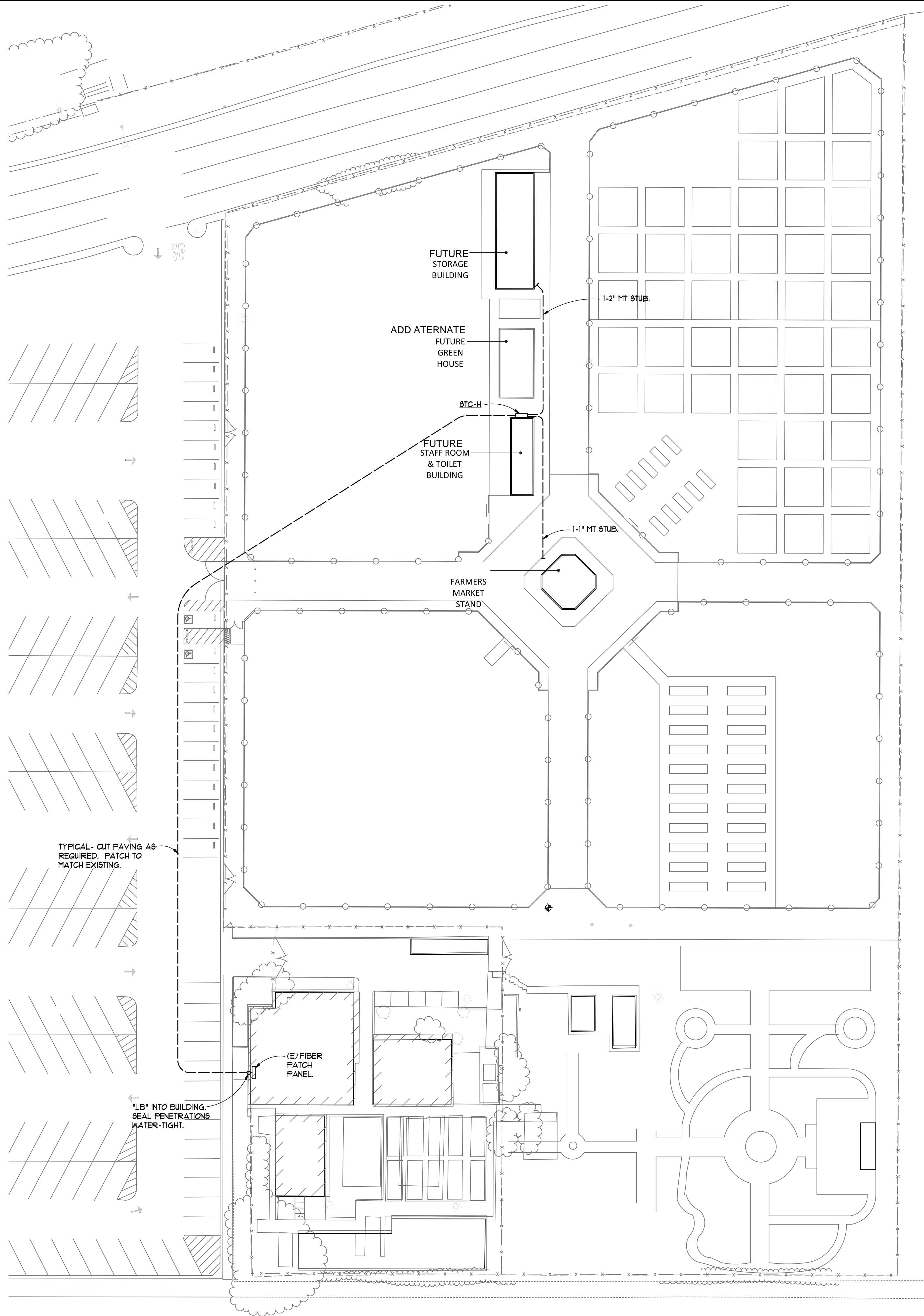












**A**  
**E1.2A** **SIGNAL SITE PLAN**  
SCALE: 1" = 30'-0"

ARCHITECT

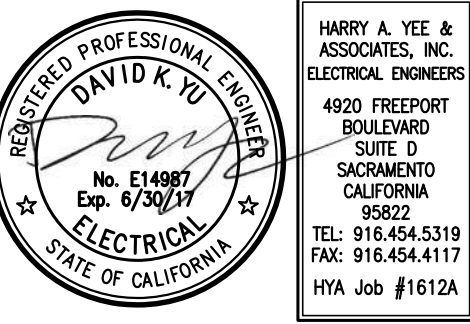
**MADI**  
ARCHITECTURE + PLANNING  
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SAN FRANCISCO, CA 94105  
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SANTA CRUZ, CA 95060  
TEL: 800.725.0571

OWNER



SOLANO COMMUNITY COLLEGE DISTRICT  
4000 Suisun Valley Rd, Fairfield, CA 94534

CONSULTANT



PROFESSIONAL STAMP:

PROJECT:

**HORTICULTURE &  
PLANT SCIENCE  
INSTITUTE PHASE II:  
MODULAR  
BUILDINGS**  
4000 Suisun Valley Rd,  
Fairfield, CA 94534

REVISIONS

REF	DESCRIPTION	DATE
-	DSA SUBMITTAL	3/7/16

PROJECT CODE: SCCD-04

START DATE:

DRAWN BY: HW-DB

CHECKED BY: DY

SHEET NAME:

**SIGNAL  
SITE PLAN**

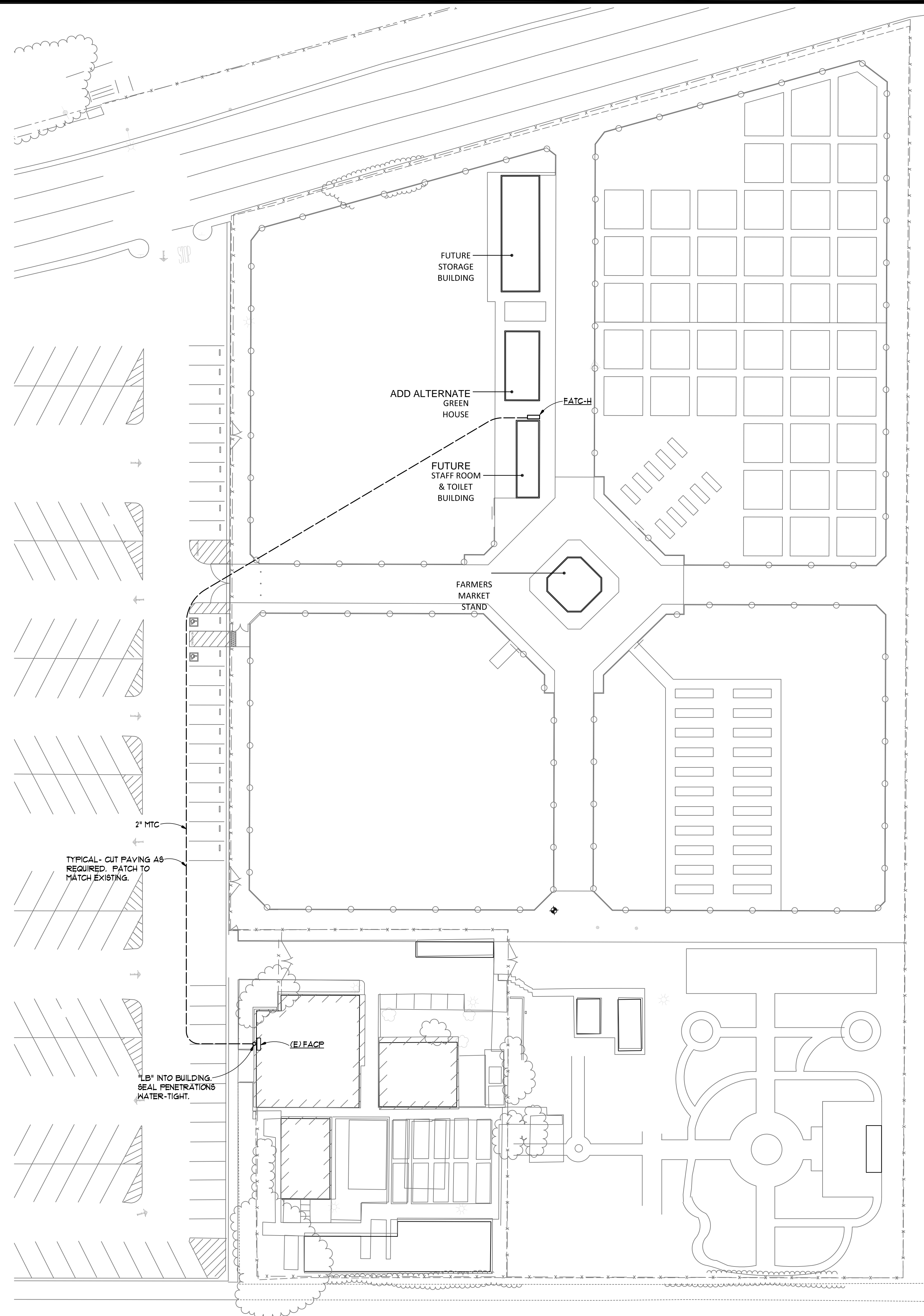
DSA APPROVAL STAMP:

SHEET NUMBER:

**E1.2A**  
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**FIRE ALARM SITE PLAN**  
SCALE: 1" = 30'-0"

ARCHITECT

**MADI**  
ARCHITECTURE + PLANNING  
333 1ST STREET, SUITE C  
SAN FRANCISCO, CA 94105  
303 POTRERO STREET, SUITE 7B  
SANTA CRUZ, CA 95060  
TEL: 800.725.0571

OWNER

**SOLANO**  
COMMUNITY COLLEGE  
SOLANO COMMUNITY COLLEGE DISTRICT  
4000 Suisun Valley Rd, Fairfield, CA 94534

CONSULTANT

**HARRY A. YEE & ASSOCIATES, INC.**  
ELECTRICAL ENGINEERS  
4920 FREEPORT BOULEVARD  
SUITE D  
SACRAMENTO, CALIFORNIA 95822  
TEL: 916.454.5319  
FAX: 916.454.4117  
HTA Job #1612A

PROFESSIONAL STAMP:

PROJECT:  
**HORTICULTURE & PLANT SCIENCE INSTITUTE PHASE II: MODULAR BUILDINGS**  
4000 Suisun Valley Rd,  
Fairfield, CA 94534

REVISIONS		
REF	DESCRIPTION	DATE
-	DSA SUBMITTAL	3/7/16

PROJECT CODE: SCCD-04  
START DATE: .  
DRAWN BY: HW-DB  
CHECKED BY: DY  
SHEET NAME:

**FIRE ALARM SITE PLAN**

DSA APPROVAL STAMP:

SHEET NUMBER:

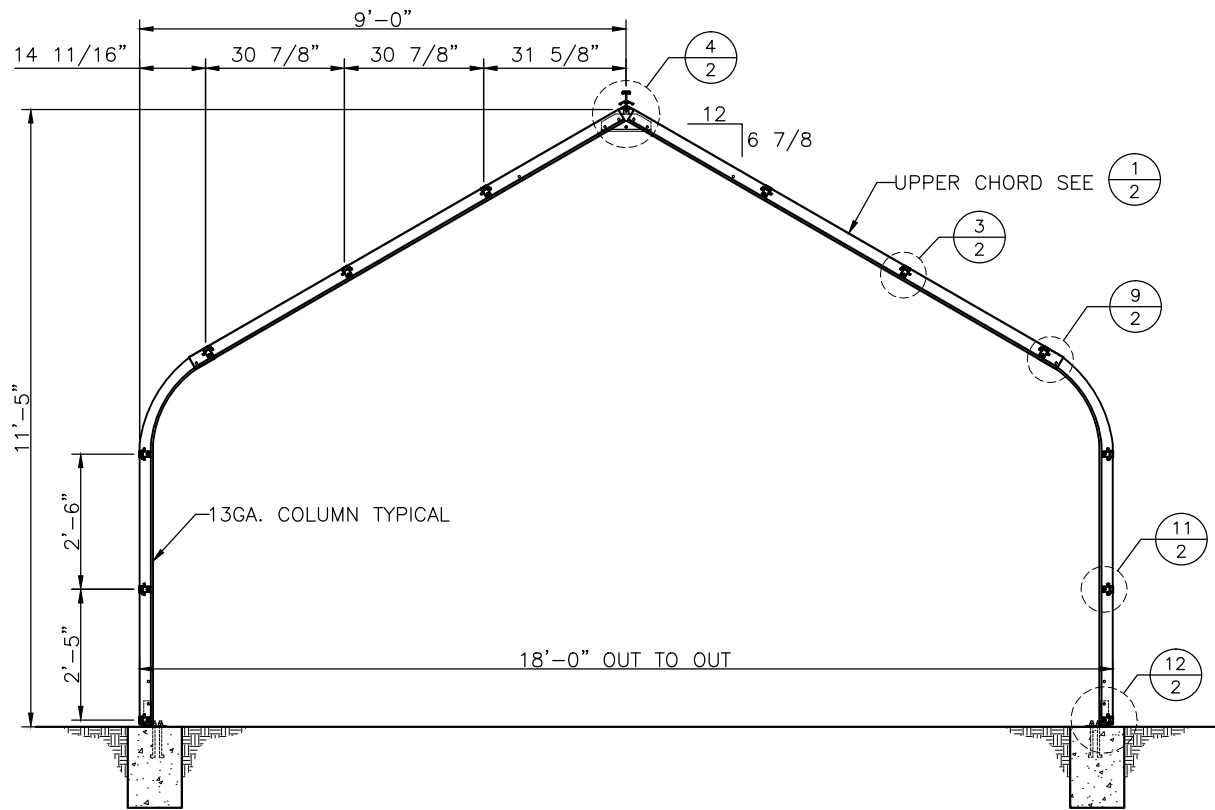
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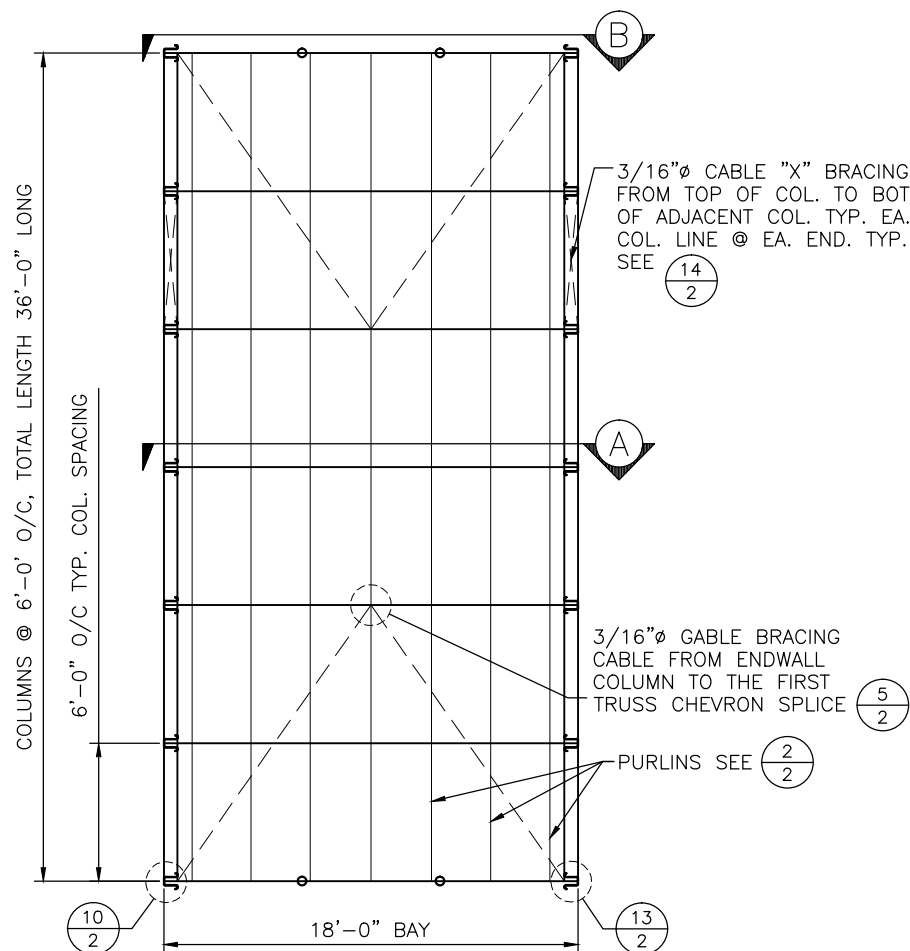






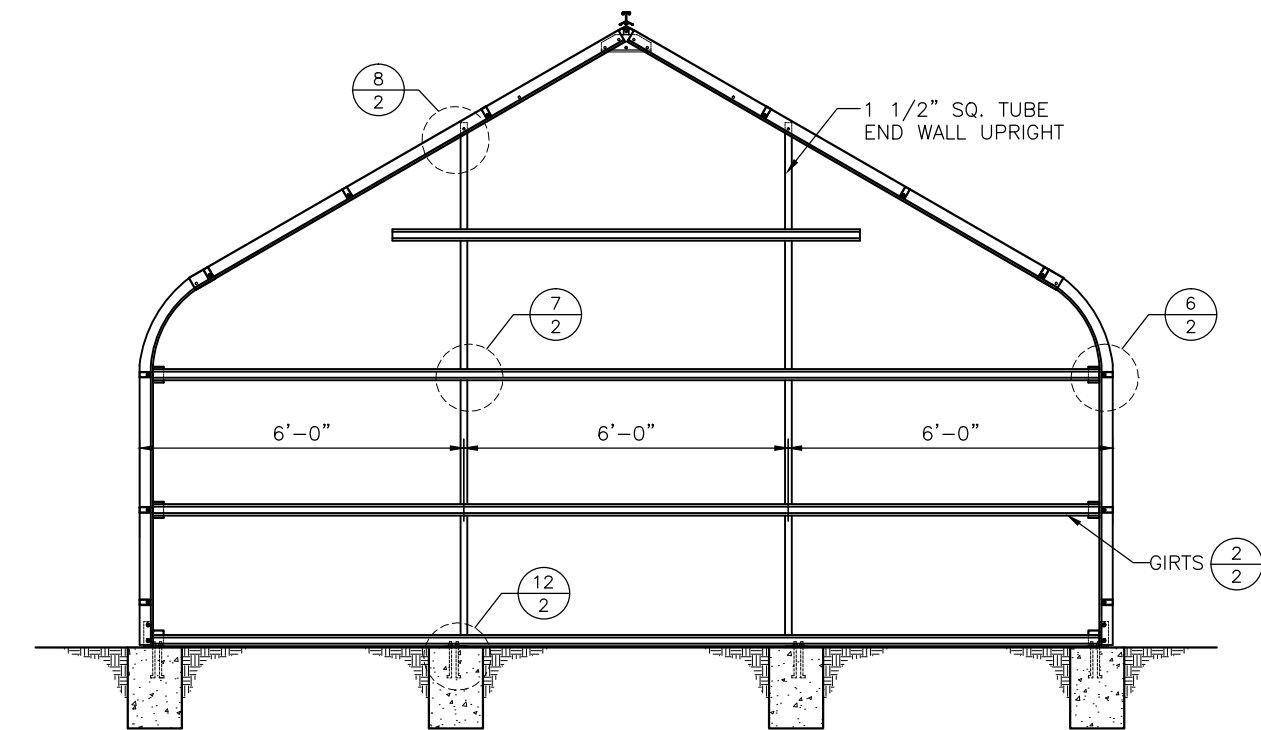
TYPICAL SECTION

SCALE: 3/8" = 1'



TYPICAL PLAN VIEW

SCALE: NTS



TYPICAL ENDWALL

SCALE: 3/8" = 1'

ADD ALTERNATE-1

PROVIDE CONLEY'S OR EQUAL

CONCRETE NOTES:

1. ALL CONCRETE SHALL WITHSTAND 2500 LBS. PER SQUARE INCH ULTIMATE COMPRESSIVE STRESS AT 28 DAYS.
2. CONTRACTOR SHALL INFORM CONLEY'S MANUFACTURING & SALES OF ANY DISCREPANCIES, OMISSIONS, OR ERRORS ON THE PLANS BEFORE BEGINNING CONSTRUCTION, OTHERWISE, IT SHALL BE DONE AS INTENDED BY THE ENGINEER.
3. THE ENGINEER AND/OR CONLEY'S MANUFACTURING & SALES ASSUMES NO RESPONSIBILITY FOR CONSTRUCTION SUPERVISION OR DEVIATION FROM THESE PLANS WITHOUT PRIOR WRITTEN APPROVAL.
4. ALL CONSTRUCTION SHALL COMPLY WITH THE C.B.C. LATEST EDITION AS AMENDED BY THE LOCAL AGENCY HAVING JURISDICTION.
5. DO NOT SCALE DRAWINGS. WRITTEN DIMENSIONS ON DRAWINGS SHALL TAKE PRECEDENCE OVER SCALED DIMENSIONS.
6. ANY ENGINEERING DESIGN PROVIDED BY OTHERS MUST BE SUBMITTED FOR REVIEW AND SHALL BEAR THE STAMP AND SIGNATURE OF A REGISTERED ENGINEER.
7. ALL PLUMBING, ELECTRICAL OR MISCELLANEOUS STUB OUT SHALL BE A MINIMUM OF NINE (9) INCHES CLEAR OF THE OUTSIDE CONCRETE IN ORDER TO CLEAR THE WALLS.
8. FOOTINGS SHALL BE CENTERED ON THE CENTERLINE OF THE COLUMN ABOVE UNLESS OTHERWISE NOTED.
9. ALL FOOTINGS SHALL BEAR AGAINST FIRM NATURAL UNDISTURBED SOIL OR CERTIFIED COMPACTED FILL. SOIL BEARING PRESSURE EQUAL TO 1500 PSF.
10. THE MINIMUM REQUIREMENTS AND LOCAL FROST LINE REQUIREMENTS MAY SUPERSEDE THE DESIGN CALL OUTS. CONTACT THE LOCAL BUILDING DEPARTMENT FOR MINIMUM DEPTH REQUIREMENTS.

\* BUILDINGS WITH SNOW LOADS ARE DESIGNED AS HEATED BUILDINGS

STEEL NOTES:

1. ALL CONSTRUCTION TO COMPLY WITH THE LATEST EDITION OF THE C.B.C. AND A.I.S.C.
2. ALL MACHINE BOLTS TO COMPLY WITH A.S.T.M. A-307\*. HOLES SHALL BE BOLT DIAMETER PLUS 1/16". (\* UNLESS OTHERWISE NOTED)
3. ALL HOT ROLLED OR COLD ROLLED SHEETS AND STRIPS USED IN THE FABRICATION OF COLD FORMED STRUCTURAL MEMBERS SHALL HAVE A MINIMUM YIELD STRENGTH OF 55 K.S.I.
4. LIGHT GAGE - COLD FORMED STRUCTURAL STEEL MEMBERS SHALL CONFORM TO A.S.T.M. SPEC. A-500 GRADE "D" (Fy=50 K.S.I.), UNLESS OTHERWISE NOTED.
5. ALL STRUCTURAL STEEL MEMBERS SHALL BE GALVANIZED.
6. ROUND TUBES SHALL CONFORM TO A.S.T.M. SPEC. A-500 GRADE "C" (Fy=46K.S.I.).
7. CABLES SHALL BE OF AIRCRAFT TYPE CABLE WITH THE FOLLOWING BREAKING STRENGTHS: 1/8"φ = 1,700 LBS., 3/16"φ = 4,200 LBS., 1/4"φ = 7,000 LBS.

BUILDING SPECIFICATIONS:

THIS STRUCTURE HAS BEEN DESIGNED AND DETAILED FOR THE LOADS AND CONDITIONS SHOWN ON THESE DRAWINGS. ANY ALTERATIONS TO THE STRUCTURAL SYSTEM OR REMOVAL OF ANY COMPONENT PARTS OR THE ADDITION OF OTHER CONSTRUCTION MATERIALS OR LOADS MUST BE DONE UNDER THE ADVICE AND DIRECTION OF A REGISTERED ARCHITECT, CIVIL OR STRUCTURAL ENGINEER. CONLEY'S MANUFACTURING & SALES WILL ASSUME NO RESPONSIBILITY FOR ANY LOADS NOT INDICATED.

THIS METAL BUILDING IS DESIGNED WITH CONLEY'S MANUFACTURING & SALES DESIGN PRACTICES WHICH ARE BASED ON PERTINENT PROCEDURES AND RECOMMENDATIONS OF THE FOLLOWING ORGANIZATIONS AND CODES, AND ARE ACCEPTED PRACTICES IN THE LOW RISE METAL AND AGRICULTURAL BUILDING INDUSTRY.

AMERICAN INSTITUTE OF STEEL CONSTRUCTION:

"STEEL CONSTRUCTION MANUAL" 13TH EDITION.  
2005 A.I.S.C. (M.B.M.A.) "SERVICEABILITY" STANDARDS WILL BE USED FOR THIS DESIGN.

AMERICAN IRON AND STEEL INSTITUTE:

2007 EDITION: NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS.

INTERNATIONAL CONFERENCE OF BUILDING OFFICIALS:

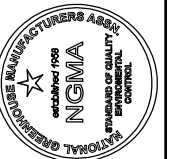

"CALIFORNIA BUILDING CODE" 2013 EDITION

AMERICAN WELDING SOCIETY:

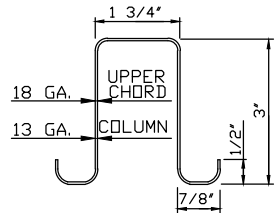
"STRUCTURAL WELDING CODE" A.W.S D1.1-10

METAL BUILDING MANUFACTURER'S ASSOCIATION:

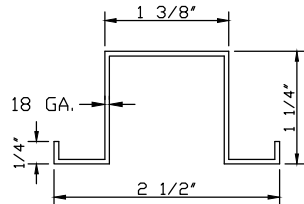
"METAL BUILDING SYSTEMS MANUAL" 2006

REVISIONS				
				
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DATE	12/30/14	SCALE	N.T.S.	JOB NO.
				—
				DRAWN BY
				AGM
				APPROVED
TITLE				
18' X 36' HOBBY HOUSE				
ENGINEER				
—				
LOADING				
12LL-100-C				
DRAWING NO.				
FM-01				
DRAWING SET				
—				
SHEET				
1 OF 2				
CERTIFICATION				

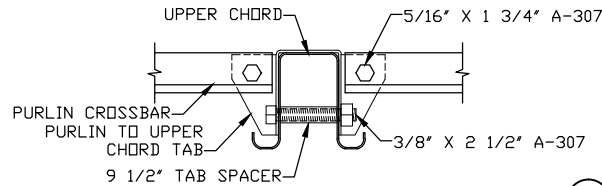




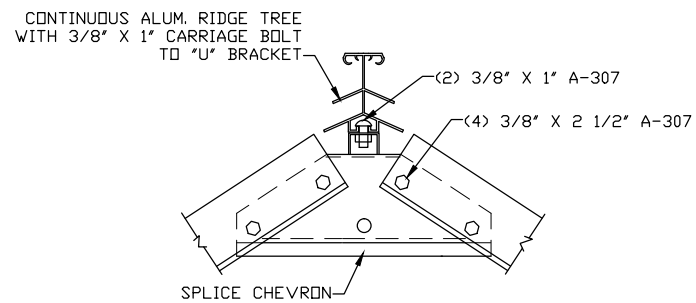
UPPER CHORD / COLUMN ①  
SCALE: NTS



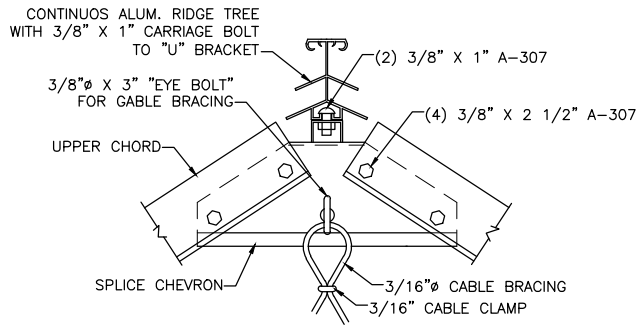
PURLIN / GIRT ②  
SCALE: NTS



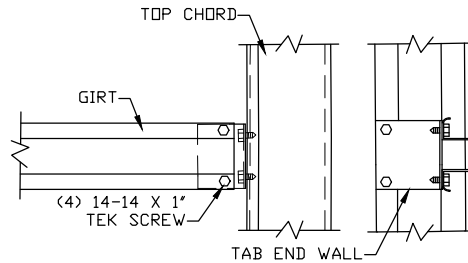
PURLIN / GIRT TO UPPER CHORD ③  
SCALE: NTS



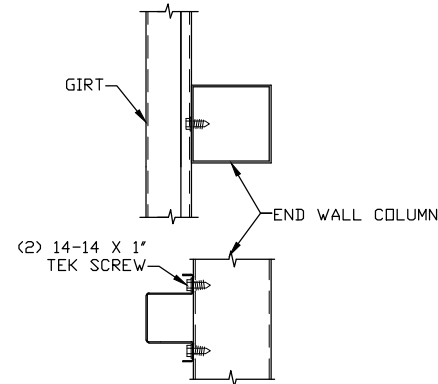
CHEVRON CONN. TO UPPER CHORD ④  
SCALE: NTS



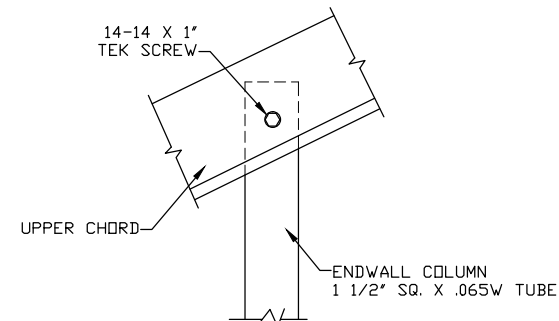
CHEVRON CONN. TO UPPER CHORD ⑤  
SCALE: NTS



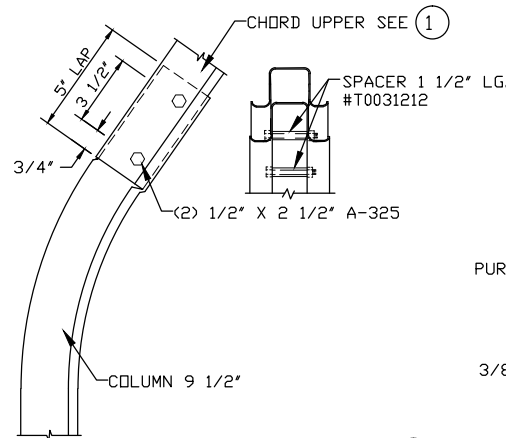
SIDEWALL AND ENDWALL UPRIGHT CONN. ⑥  
SCALE: NTS



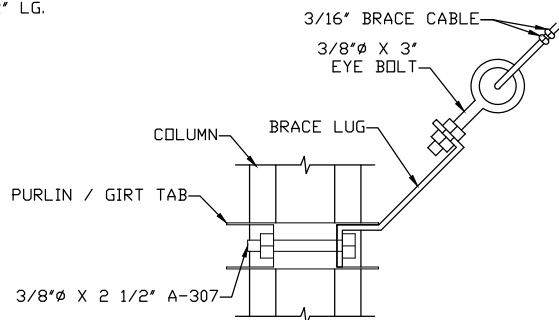
GIRT CONN. AT ENDWALL COLUMN ⑦  
SCALE: NTS



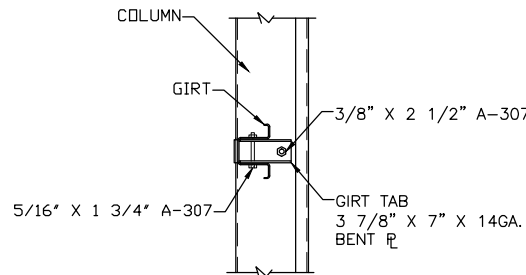
END WALL UPRIGHT AT CHORD ⑧  
SCALE: NTS



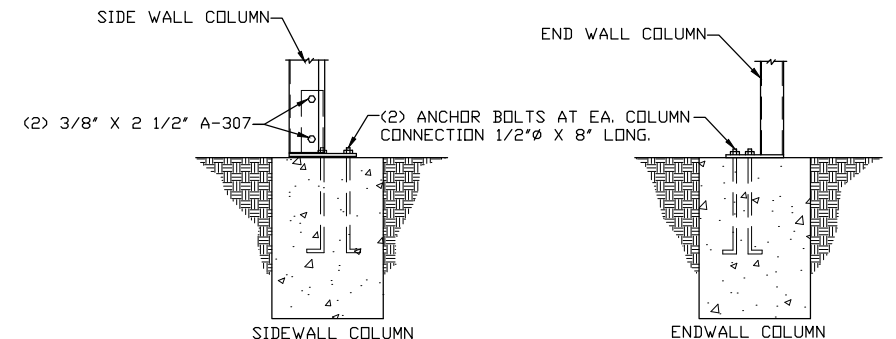
COLUMN CONN. TO UPPER CHORD ⑨  
SCALE: NTS



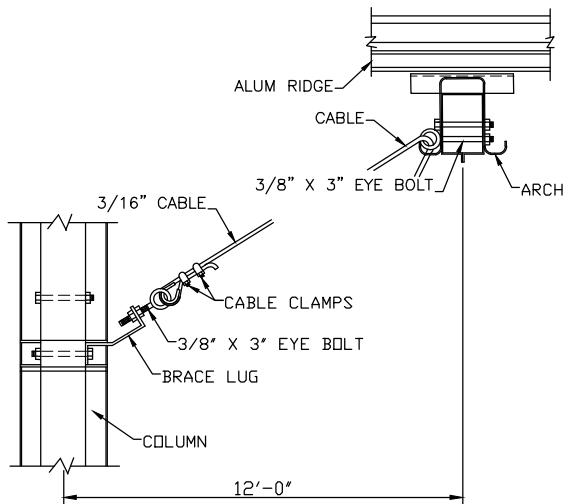
BRACE CABLE CONNECTION ⑩  
SCALE: NTS



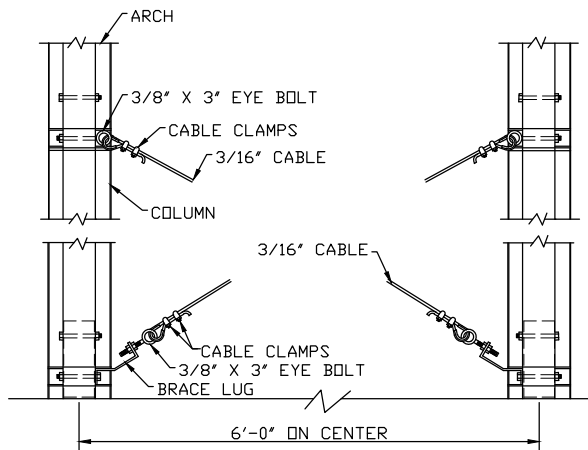
GIRT AT COLUMN CONNECTION ⑪  
SCALE: NTS



SIDEWALL AND ENDWALL UPRIGHT CONN. ⑫  
SCALE: NTS



CABLE BRACING AT RIDGE ⑬  
SCALE: NTS



COLUMN \"X\" BRACING ⑭  
SCALE: NTS

**ADD ALTERNATE-1**  
PROVIDE CONLEY'S OR EQUAL

REVISIONS



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DATE	12/30/14
SCALE	N.T.S.
JOB NO.	—
DRAWN BY	AGM
APPROVED	

TITLE	HOBBY HOUSE DETAILS
ENGINEER	—
LOADING	12LL-100-C
DRAWING NO.	FM-02
DRAWING SET	—
SHEET	2 OF 2
CERTIFICATION	



EXAMPLE FORM DSA 103

NOTE: THE EXAMPLE FORM DSA-103 SHOWN ON THIS SHEET IS FOR ILLUSTRATION PURPOSES ONLY TO ASSIST IN THE COMPLETION OF FUTURE PROJECT SPECIFIC FORM DSA-103'S. A FORM DSA-103 IS TO BE COMPLETED FOR EACH APPLICATION THAT THIS PC IS BEING INCORPORATED INTO AND THE EXAMPLE FORM DSA-103 IS TO BE CROSSED OUT ON THIS DRAWING.

Note: References are to the 2013 edition of the California Building Code (CBC) unless otherwise noted.

REQUIRED	TEST OR SPECIAL INSPECTION	TYPE	PERFORMED BY	
-	SOILS			
	1. GENERAL:	Table 1705A.6		
X	a. Verify that: <ul style="list-style-type: none"><li>• site has been prepared properly prior to placement of controlled fill and/or excavations for foundations,</li><li>• foundation excavations are extended to proper depth and have reached proper material, and</li><li>• materials below footings are adequate to achieve the design bearing capacity.</li></ul>	Periodic	GE*	* By geotechnical engineer or his or her qualified representative. Use PI in lieu of GE if there is no GE for the site specific project.
-	2. COMPACTED FILLS:	Table 1705A.6		
X	a. Perform qualification testing of fill materials.	Test	Lab*	* Under the supervision of the geotechnical engineer.
X	b. Verify use of proper materials and inspect lift thicknesses, placement, and compaction during placement of fill.	Continuous	GE*	* By geotechnical engineer or his or her qualified representative. Use PI in lieu of GE if there is no GE for the site specific project.
X	c. Test compaction of fill.	Test	Lab*	* Under the supervision of the geotechnical engineer.
-	4. CAST-IN-PLACE DEEP FOUNDATIONS (PIERS):	Table 1705A.7		
X	a. Inspect drilling operations and maintain complete and accurate records for each pier.	Continuous	GE*	* By geotechnical engineer or his or her qualified representative. Use PI in lieu of GE if there is no GE for the site specific project.
X	b. Verify locations of piers.	Continuous	PI	
X	c. Confirm pier diameters, plumbness, bell diameters (if applicable), lengths, and embedment into bedrock (if applicable). Record concrete or grout volumes.	Continuous	GE*	* By geotechnical engineer or his or her qualified representative. Use PI in lieu of GE if there is no GE for the site specific project.
X	d. Confirm adequate end strata bearing capacity.	Test	Lab*	* Under the supervision of the geotechnical engineer.
X	e. Concrete piers	Provide tests and inspections per CONCRETE section below		
-	CONCRETE	Table 1705A.3		
-	7. CAST IN PLACE CONCRETE			
	Material Verification and Testing:			
X	a. Verify use of required design mix.	Periodic	SI & PI*	* To be performed by batch-plant special inspector and project inspector.
X	c. Perform slump, temperature, and (where required) air content tests.	Test	Lab	ASTM C172, ASTM C31.
X	d. Test concrete (compression).	Test	Lab	ACI 318 Section 5.6 and 1905A.1.2 (1913.3.1)*, ASTM C39.
	Inspection:			
X	e. Batch plant inspection	Continuous	SI	1705A.3.2. If approved by DSA, batch plant inspection may be reduced to periodic if plant complies with 1705A.3.3, Item 1, and requires first batch inspection, weighmaster, and batch tickets.
X	g. Inspect placement of formwork, reinforcing steel, embedded items and concrete. Inspect curing and form removal.	Continuous	PI*	* May be performed by a special inspector when specifically approved by DSA.
+	MASONRY	TMS 402-11/ACI 530-11/ASCE 5-11 Table 1.19.3		
-	STEEL	Table 1705A.2.1		
-	17. STRUCTURAL STEEL AND COLD-FORMED STEEL USED FOR STRUCTURAL PURPOSES			
	Material Verification:			
X	a. Verify that all materials are appropriately marked and that: <ul style="list-style-type: none"><li>• Mill certificates indicate material properties that comply with requirements.</li><li>• Material sizes, types and grades comply with requirements.</li></ul>	Periodic	*	* By special inspector when performed off-site; by project inspector for steel shipped directly to project site without welding or fabrication.
X	b. Test unidentified materials	Test	Lab	2203A.1 (2203.1*). ASTM A370.
X	c. Examine seam welds of structural tubes and pipes	Periodic	SI*	* DSA IR 17-3.
	Inspection:			
X	d. Verify member locations, bracing and all details constructed in the field.	Continuous	PI	
X	e. Verify stiffener locations, connection tab locations and all construction details fabricated in the shop.	Periodic	SI	
-	18. HIGH STRENGTH BOLTS:			
	Material Verification of High-Strength Bolts, Nuts, and Washers:			
X	a. Verify identification markings and manufacturer's certificates of compliance conform to ASTM standards specified in the DSA approved documents.	Periodic	SI	DSA IR 17-9
X	b. Test high-strength bolts, nuts and washers.	Test	Lab	2213A.1 (2212.6.1*). ASTM F606, A370. DSA IR 17-8
	Inspection of High-Strength Bolt Installation:			
X	c. Bearing-type ("snug tight") connections.	Periodic	SI*	DSA IR 17-9
	19. WELDING:			DSA IR 17-3, AWS D1.1 and AWS D1.8 (AWS D1.3 for cold formed steel).
	Verification of Materials, Equipment, Welders, etc:			
X	a. Verify weld filler material identification markings per AWS designation listed on the DSA approved documents and the WPS.	Periodic	SI	
X	b. Verify weld filler material manufacturer's certificate of compliance.	Periodic	SI	
X	c. Verify WPS, welder qualifications and equipment.	Periodic	SI	DSA IR 17-3.
-	19.1 SHOP WELDING:			
X	a. Inspect groove, multi-pass, and fillet welds > 5/16"	Continuous	SI	Per AISC 360 (and AISC 341 as applicable), DSA IR 17-3.
X	b. Inspect single-pass fillet welds ≤ 5/16"	Periodic	SI	Per AISC 360 (and AISC 341 as applicable), DSA IR 17-3.
+	WOOD			
+	OTHER			

1 Soils testing and inspection: Geotechnical Verified Report - Form DSA-293

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KEY to Columns

1 Type -

Continuous – Indicates that a continuous special inspection is required

Periodic – Indicates that a periodic special inspection is required

Test – Indicates that a test is required

2 Performed By -

GE – Indicates that the special inspection is to be performed by a registered geotechnical engineer or his or her authorized representative

Lab – Indicates that the test or inspection is to be performed by a testing laboratory accepted in the DSA laboratory Evaluation and Acceptance (LEA) Program. See section 4-335, 2013 CCR Title 24, Part 1.

PI – Indicates that the special inspection is to be performed by the project inspector

SI – Indicates that the special inspection is to be performed by a special inspector

2013 CBC PC STRUCTURAL DESIGN NOTES

DESCRIPTION	DESIGN VALUES
DEAD AND LIVE LOADS	
ROOF LIVE LOAD (L <sub>r</sub> )	20 PSF
ROOF DEAD LOAD (D)	5 PSF
ALLOWABLE SOIL PRESSURE	
DL	1000 PSF
DL+L <sub>r</sub>	1000 PSF
DL+SNOW	1000 PSF
ROOF SNOW LOAD	
GROUND SNOW LOAD (P <sub>g</sub> )	22 PSF
SLOPED ROOF SNOW LOAD (P <sub>s</sub> )	20 PSF
SNOW EXPOSURE FACTOR (C <sub>e</sub> )	1.1
SNOW IMPORTANCE FACTOR (I)	1.0
THERMAL FACTOR (C <sub>t</sub> )	1.2
FLOOD DESIGN	
FLOOD HAZARD AREA	NO
WIND DESIGN	
ULTIMATE DESIGN WIND SPEED (V <sub>ult</sub> )	130 MPH
WIND EXPOSURE FACTOR	C
TOPOGRAPHIC FACTOR (K <sub>zt</sub> )	1.0
ASCE 7-10 WIND ANALYSIS METHOD	CHAPTER 27 DIRECTIONAL PROCEDURE
VELOCITY PRESSURE EXPOSURE COEFFICIENT (K <sub>z</sub> )	0.85
NET PRESSURE COEFFICIENT	VARIES, SEE CALCULATIONS
WIND DIRECTIONALITY FACTOR (K <sub>d</sub> )	0.85
WIND VELOCITY PRESSURE (q <sub>h</sub> )	31.3 PSF
SEISMIC DESIGN	
ASCE 7-10 ANALYSIS PROCEDURE	SECTION 12.8 EQUIVALENT LATERAL FORCE PROCEDURE
SEISMIC DESIGN CATEGORY	E
SEISMIC IMPORTANCE FACTOR	1.0
SITE CLASS	D
MAPPED MCE, 5% DAMPED, SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD (S <sub>s</sub> )	1.875
SHORT PERIOD SITE COEFFICIENT (F <sub>a</sub> )	1.0
DESIGN MCE, 5% DAMPED, SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD (S <sub>ds</sub> )	1.0
MAPPED MCE, 5% DAMPED, SPECTRAL RESPONSE ACCELERATION AT 1 SECOND PERIOD (S <sub>1</sub> )	1.3
LONG PERIOD SITE COEFFICIENT (F <sub>v</sub> )	1.5
DESIGN, 5% DAMPED, SPECTRAL RESPONSE ACCELERATION AT 1 SECOND PERIOD (S <sub>1d</sub> )	1.3
HORIZONTAL OR VERTICAL IRREGULARITY TYPES	NONE

BUILDING DATA

CONSTRUCTION CLASSIFICATION	TYPE II-B
OCCUPANCY CLASSIFICATION	A-2
RISK CATEGORY	II
NUMBER OF STORIES	1

SHEET INDEX

AMERICANA SHELTERS

☒ G.1 AMERICANA SHELTERS DESIGN NOTES, EXAMPLE FORM DSA 103

10' AND 16' ILLINI SHELTER

☐ IT.0 ILLINI SHELTER DESIGN NOTES, EXAMPLE FORM DSA 103

☐ IT.1 ILLINI SHELTER PLANS, SECTIONS AND DETAILS

20' MERAMEC SHELTERS

☐ MT20.0 20' MERAMEC SHELTER DESIGN NOTES, EXAMPLE FORM DSA 103

☐ MT20.1 20' MERAMEC SHELTER PLANS, SECTIONS AND DETAILS

30' MERAMEC SHELTERS

☐ MT30.0 30' MERAMEC SHELTER DESIGN NOTES, EXAMPLE FORM DSA 103

☐ MT30.1 30' MERAMEC SHELTER PLANS, SECTIONS AND DETAILS

16' NAVAJO SHELTERS

☐ NT16.0 16' NAVAJO SHELTER DESIGN NOTES, EXAMPLE FORM DSA 103

☐ NT16.1 16' NAVAJO SHELTER PLANS, SECTIONS AND DETAILS

20' NAVAJO SHELTERS

☐ NT20.0 20' NAVAJO SHELTER DESIGN NOTES, EXAMPLE FORM DSA 103

☐ NT20.1 20' NAVAJO SHELTER PLANS AND ELEVATIONS

☐ NT20.2 20' NAVAJO SHELTER SECTIONS AND DETAILS

24' NAVAJO SHELTERS

☐ NT24.0 24' NAVAJO SHELTER DESIGN NOTES, EXAMPLE FORM DSA 103

☐ NT24.1 24' NAVAJO SHELTER PLANS AND ELEVATIONS

☐ NT24.2 24' NAVAJO SHELTER SECTIONS AND DETAILS

30' NAVAJO SHELTERS

☒ NT30.0 30' NAVAJO SHELTER DESIGN NOTES, EXAMPLE FORM DSA 103

☒ NT30.1 30' NAVAJO SHELTER PLANS AND ELEVATIONS

☒ NT30.2 30' NAVAJO SHELTER SECTIONS AND DETAILS

36' NAVAJO SHELTERS

☐ NT36.0 36' NAVAJO SHELTER DESIGN NOTES, EXAMPLE FORM DSA 103

☐ NT36.1 36' NAVAJO SHELTER PLANS AND ELEVATIONS

☐ NT36.2 36' NAVAJO SHELTER SECTIONS AND DETAILS

40' NAVAJO SHELTERS

☐ NT40.0 40' NAVAJO SHELTER DESIGN NOTES, EXAMPLE FORM DSA 103

☐ NT40.1 40' NAVAJO SHELTER PLANS AND ELEVATIONS

☐ NT40.2 40' NAVAJO SHELTER SECTIONS AND DETAILS

7' SHAWNEE SHELTER

☐ ST7.0 SHAWNEE SHELTER DESIGN NOTES, EXAMPLE FORM DSA 103

☐ ST7.1 SHAWNEE SHELTER PLANS, SECTIONS AND DETAILS

GENERAL NOTES

I. SHELTER DESIGN

A. THE STRUCTURAL DESIGN OF THE COMPONENTS AND CONNECTIONS OF THIS SHELTER ARE SUFFICIENT FOR EAVE HEIGHTS RANGING FROM 7' UP TO 12' TALL.

B. REQUIRED EAVE HEIGHT FOR EACH SITE SHALL BE DETERMINED BY OWNER.

C. THIS SHELTER HAS BEEN DESIGNED AS AN OPEN STRUCTURE. THE ADDITION OF ANY ENCLOSURE DIRECTLY ATTACHED TO THE SHELTER, SUCH AS WALLS, INSECT MESH, OR SHADE SCREENS, SHALL BE PROHIBITED AS INCREASED WIND FORCES MAY RESULT.

II. DESIGN AND CONSTRUCTION STANDARDS

A. THE DESIGN OF THIS STRUCTURE IS IN CONFORMANCE WITH THE FOLLOWING STANDARDS AND ALL PHASES OF CONSTRUCTION SHALL BE PERFORMED IN ACCORDANCE WITH THE FOLLOWING STANDARDS.

1. 2013 CALIFORNIA ADMINISTRATIVE CODE (CAC) (PART 1, TITLE 24, CCR)

2. 2013 CALIFORNIA BUILDING CODE (CBC), VOLUMES 1 AND 2 (PART 2, TITLE 24, CCR)

3. 2012 INTERNATIONAL BUILDING CODE WITH 2013 CALIFORNIA AMENDMENTS

4. 2013 CALIFORNIA ELECTRICAL CODE (PART 3, TITLE 24, CCR)

5. 2013 CALIFORNIA MECHANICAL CODE (CMC) (PART 4, TITLE 24, CCR)

6. 2012 CALIFORNIA PLUMBING CODE (CPC) (PART 5, TITLE 24, CCR)

7. 2012 CALIFORNIA FIRE CODE (CFC) (PART 9, TITLE 24, CCR)

8. 2013 CALIFORNIA GREEN BUILDING STANDARDS CODE (PART 11, TITLE 24, CCR)

9. 2013 CALIFORNIA REFERENCED STANDARDS CODE (PART 12, TITLE 24, CCR)

10. NFPA 13 - 2013

11. NFPA 72 - 2013

III. CONSTRUCTION CHANGES

A. CHANGES TO THE APPROVED PLANS AND SPECIFICATIONS SHALL BE MADE BY AN ADDENDA OR CONSTRUCTION CHANGE DOCUMENT APPROVED BY THE DIVISION OF THE STATE ARCHITECT AS REQUIRED BY PART 1, TITLE 24, C.C.R.

IV. FOUNDATION

A. THE FOUNDATION SHALL REST ON SOUND SOIL THAT IS FREE OF ORGANIC AND DELETERIOUS MATERIALS AND CAPABLE OF SUPPORTING 1000 PSF VERTICAL BEARING PRESSURE.

B. FOR LATERAL LOADING, THE FOUNDATION HAS BEEN DESIGNED TO THE MINIMUM LATERAL BEARING VALUE IN CBC TABLE 1808A.2. THIS IS 100 PSF/FT LATERAL BEARING.

C. FOUNDATION DESIGN SHOWN IS BASED ON SOIL CONDITIONS GIVEN IN NOTES A AND B. ABOVE. OWNER SHALL VERIFY ACTUAL SOIL CONDITIONS AT EACH JOB SITE AND ANY REQUIRED ADJUSTMENTS TO THE FOOTING DESIGN SHALL BE DESIGNED BY OTHERS.

V. CONCRETE

A. COMPRESSION STRENGTH OF ALL REINFORCED CONCRETE SHALL NOT BE LESS THAN 4000 PSI AT 28 DAYS.

B. REINFORCING BARS SHALL BE DEFORMED BARS CONFORMING TO THE REQUIREMENTS OF MINIMUM ASTM A615 GRADE 40 FOR #4 AND SMALLER BARS AND GRADE 60 FOR BARS LARGER THAN #4.

C. MINIMUM CONCRETE CLEAR COVER FOR REINFORCING BARS SHALL BE 3".

D. A CONCRETE MIX DESIGN IN ACCORDANCE WITH CBC SECTION CHAPTER 19A SHALL BE PERFORMED AND STAMPED BY A CIVIL ENGINEER LICENSED IN THE STATE OF CALIFORNIA. THE CONCRETE MIX DESIGN SHALL BE SUBMITTED TO THE INSPECTOR OF RECORD PRIOR TO CONSTRUCTION.

E. THE MIX DESIGN SHALL MEET THE CRITERIA HEREIN AND SHALL BE PROPER FOR LOCAL CONDITIONS INCLUDING, BUT NOT LIMITED TO, FREEZING AND THAWING EXPOSURE, CHEMICAL AND SALT EXPOSURE, AND SOIL CORROSIVITY WHERE SUCH PROBLEMS EXIST.

F. NON-SHRINK GROUT OR DRY PACK SHALL BE A PREMIXED, NONMETALLIC FORMULA WITH A MINIMUM COMPRESSIVE STRENGTH OF 7000 PSI AT 28 DAYS AND HAVING THE FOLLOWING CHARACTERISTICS: NO SHRINKAGE AFTER PLACEMENT OR EXPANSION AFTER SET (ASTM C1090), ONE DAY COMPRESSIVE STRENGTH OF AT LEAST 3000 PSI (ASTM C109) AND INITIAL SET TIME OF NOT LESS THAN 45 MINUTES (ASTM C191). PROVIDE "HI-FLOW GROUT" OR "DRY PACK GROUT" BY EUCLID, OR AN APPROVED EQUAL.

VI. STRUCTURAL STEEL

A. STEEL PLATE SHALL CONFORM TO THE REQUIREMENTS OF ASTM A36.

B. HOLLOW STRUCTURAL SECTIONS (HSS) SHALL CONFORM TO THE REQUIREMENTS OF ASTM A500, GRADE B.

C. ALL STRUCTURAL STEEL SHALL BE IDENTIFIED BY MILL CERTIFICATE.

D. HIGH STRENGTH BOLTS (HSB) SHALL BE GALVANIZED AND SHALL CONFORM TO THE REQUIREMENTS OF ASTM A325-N. HIGH STRENGTH BOLTS SHALL BE TIGHTENED TO A SNUG TIGHT CONDITION PLUS AN ADDITIONAL HALF TURN.

E. ALL HIGH STRENGTH BOLTS SHALL HAVE CERTIFICATION.

F. WELDING SHALL CONFORM TO THE REQUIREMENTS OF THE AMERICAN WELDING SOCIETY'S SPECIFICATION FOR THE MATERIAL BEING WELDED. ALL WELDING SHALL BE PERFORMED BY AWS CERTIFIED WELDERS.

G. WELD ELECTRODES SHALL BE E70XX AND SHALL CONFORM TO THE REQUIREMENTS OF AWS D1.8-6.3 FOR DEMAND CRITICAL WELDS.

H. ALL WELDING SHALL BE APPROVED BY AN AWS CERTIFIED INSPECTOR.

I. STEEL FRAMING SHALL BE COATED WITH ANTI-GRAFFITI POLYESTER TGIC POWDER COAT FINISH MEETING AAMA 2604-02 SPECIFICATIONS.

J. SHOP DRAWINGS OF ALL STRUCTURAL STEEL SHALL BE SUBMITTED TO HYTTINEN ENGINEERING FOR APPROVAL PRIOR TO FABRICATION.

K. ALL BOLT HOLE DIAMETERS SHALL BE EQUAL TO THE BOLT DIAMETER PLUS 1/16" U.N.O. BOLT HOLES FOR ANCHOR BOLTS SHALL BE EQUAL TO THE BOLT DIAMETER PLUS 1/8".

L. ANCHOR BOLTS SHALL CONFORM TO ASTM F1554, GRADE 36 AND SHALL BE HOT DIP GALVANIZED.

VII. ALUMINUM

A. INTERLOCKING SEAM ALUMINUM ROOF DECK SHALL BE ROLL FORMED FROM ALUMINUM ALLOY 3004-H151 AND SHALL CONFORM TO THE DECK PROFILE SHOWN ON THE DRAWINGS.

B. ALUMINUM ROOF DECK SHALL BE COATED WITH HEAT REFLECTIVE BASF ULTRA-COOL COATING OR APPROVED EQUAL.

C. EXTRUDED ALUMINUM RIDGE CAP SHALL BE FABRICATED FROM ALUMINUM ALLOY 6105-T5 AND SHALL CONFORM TO THE REQUIREMENTS SHOWN ON THE DRAWINGS.

D. EXTRUDED ALUMINUM FASCIA SHALL BE FABRICATED FROM ALUMINUM ALLOY 6063-T5. EXTRUDED ALUMINUM GUTTER SHALL BE FABRICATED FROM ALUMINUM ALLOY 6105-T5. ALUMINUM COMPONENTS SHALL CONFORM TO THE REQUIREMENTS SHOWN ON THE DRAWINGS.

E. EXTRUDED ALUMINUM RIDGE CAP, GUTTER, AND FASCIA SHALL BE COATED WITH ANTI-GRAFFITI POLYESTER TGIC POWDER COAT FINISH MEETING AAMA 2604-02 SPECIFICATIONS.

VIII. SCREWS

A. SCREWS SHALL BE HILTI KWIK-PRO SELF DRILLING SCREWS WITH BOND SEAL WASHERS PER ICC ESR-2196 OR APPROVED EQUAL.

B. SCREWS ATTACHING TO STEEL SHALL BE 12-24 HEX WASHER HEAD (HW#) #6 POINT SCREWS. SCREWS ATTACHING TO ALUMINUM SHALL BE 8-18 HEX WASHER HEAD (HW#) #2 POINT SCREWS.

C. ALL SCREWS SHALL BE STAINLESS STEEL OR COATED WITH ZINC.

D. THE MANUFACTURER SHALL PROVIDE A SCREW CERTIFICATION LETTER STATING THAT SCREWS PROVIDED MATCH THE SIZE AND TYPE SPECIFIED HEREIN. THE CERTIFICATION LETTER SHALL BE SUBMITTED TO THE INSPECTOR OF RECORD PRIOR TO INSTALLATION.

IX. SHOP FABRICATION AND FIELD ASSEMBLY

A. ALL STRUCTURAL STEEL AND ALUMINUM COMPONENTS SHALL BE SHOP FABRICATED SO THAT FIELD ASSEMBLY OF CONNECTIONS CAN BE PERFORMED USING ONLY BOLTING AND SCREW PLACEMENT.

X. SPECIAL INSPECTION

A. THE OWNER SHALL EMPLOY A SPECIAL INSPECTOR TO PERFORM INSPECTION OF THE CONSTRUCTION OF THIS PC IN ACCORDANCE WITH THE REQUIREMENTS OF CHAPTER 17A OF THE 2013 CALIFORNIA BUILDING CODE (PART 2, TITLE 24, C.C.R.) AND THE DIVISION OF THE STATE ARCHITECT.

XI. FIRE LIFE SAFETY

A. AN AUTOMATIC FIRE PROTECTION SYSTEM MAY BE REQUIRED FOR THIS BUILDING DEPENDING ON SITE SPECIFIC REQUIREMENTS. WHERE REQUIRED, THE AUTOMATIC FIRE PROTECTION SYSTEM SHALL BE DESIGNED BY OTHERS.

B. THE DESIGN OF THIS SHELTER IS CAPABLE OF SUPPORTING THE WEIGHT OF A FIRE SPRINKLER SYSTEM (1.5 PSF).

C. THE METAL ROOFING COMPLIES WITH FIRE CLASSIFICATION B. THIS SHELTER HAS NOT BEEN DESIGNED FOR PLACEMENT WITHIN ANY FIRE HAZARD SEVERITY ZONE.

NOTICE OF DISCLAIMER FOR STRUCTURAL ENGINEERING RESPONSIBILITY

1. PER TITLE 24, PART 1, SECTION 4-316(e) OF THE CALIFORNIA CODE OF REGULATIONS, THIS NOTICE SHALL BE GIVEN TO DSA PRIOR TO THE APPROVAL OF PLANS AND SPECIFICATIONS.

2. FOR THE SITE SPECIFIC PROJECT, ROGER HYTTINEN IS NOT THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE, UNLESS NOTED OTHERWISE.

3. FOR THE SITE SPECIFIC PROJECT, ROGER HYTTINEN'S RESPONSIBILITY IS LIMITED TO THE PREPARATION OF PLANS AND SPECIFICATIONS FOR THE SHELTER(S) OF THIS PC ONLY.

4. STRUCTURAL OBSERVATION OF CONSTRUCTION IS SPECIFICALLY EXCLUDED FROM ROGER HYTTINEN'S RESPONSIBILITY FOR THE SITE SPECIFIC PROJECT.

5. ALL CONSTRUCTION ACTIVITIES RELATED TO STRUCTURAL ENGINEERING SHALL BE DELEGATED TO A QUALIFIED ENGINEER BY THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE. THESE ACTIVITIES INCLUDE, BUT ARE NOT LIMITED TO, APPROVAL OF INSPECTOR QUALIFICATIONS, STRUCTURAL OBSERVATION OF CONSTRUCTION, REVIEW OF INSPECTION REPORTS, AND SIGNING OFF THE VERIFIED REPORT FOR COMPLETED WORK.

6. ROGER HYTTINEN WILL BE RESPONSIBLE FOR RESPONDING TO QUESTIONS PERTAINING TO THE PLANS AND SPECIFICATIONS FOR THE SHELTER(S) OF THIS PC WHICH ARISE DURING PLAN CHECK AND CONSTRUCTION.

7. IN THE EVENT THAT ROGER HYTTINEN IS REQUIRED TO PROVIDE STRUCTURAL OBSERVATION OF CONSTRUCTION, HE SHALL BE NOTIFIED IN WRITING PRIOR TO THIS REQUIREMENT BEING MADE. ALSO, HIS ANTICIPATED ADDITIONAL FEES FOR THIS ADDITIONAL WORK SHALL BE PAID IN ADVANCE, PRIOR TO ANY STRUCTURAL OBSERVATION OR CONSTRUCTION SERVICES BEING PERFORMED.

Shelter Style	Available Options		Eave Height		Recessed Anchor Bolts/Footings	Roof Downspouts	"V" plugs for bird control.
	Length	Width	Min.	Max.			
20' Meramec	30', 42'		7'	12'	Y/N	Y/N	Y/N
30' Meramec	42', 54'		7'	12'	Y/N	Y/N	Y/N
16' Navajo			7'	12'	Y/N	Y/N	Y/N
20' Navajo			7'	12'	Y/N	Y/N	Y/N
24' Navajo			7'	12'	Y/N	Y/N	Y/N
30' Navajo			7'	12'	Y/N	Y/N	Y/N
36' Navajo			7'	12'	Y/N	Y/N	Y/N
40' Navajo			7'	12'	Y/N	Y/N	Y/N
Illini	13' to 58' in 9' increments.	10', 16'	7'-6"	12'	Y/N	Y/N	Y/N
7' Shawnee	9'-8" to 55'-8" in 7'-8" increments.		7'-6"	12'	Y/N	Y/N	Y/N

PRE-CHECK (PC) DOCUMENT

CODE: 2013 CBC

A separate project application for construction is required.

OCT 23 2014

DESIGN PROFESSIONAL

FILE NO.: PC-058

APPL. NO.: 02-113840

HYTTINEN ENGINEERING

5458 Longley Lane, Suite B

Reno, Nevada 89511

(775) 826-3019 PHONE

(775) 826-3076 FAX



GENERATION STAMP

DAY OF THE STATE ARCHITECT

02 113840

DATE 10/23/14

PC

AMERICANA SHELTERS

AMERICANA BUILDING PRODUCTS

#2 Industrial Dr. - Salem, IL 62881

(800)851-0865 www.americana.com

PROJECT:

SHEET TITLES:

DESIGN NOTES, EXAMPLE FORM DSA 103

DRAWN NVGI

CHECKED R.H.

DATE 10/21/14

SCALE AS NOTED

JOB NO. 44-14

DRAWING NO. G.1

SHEET G.1

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EXAMPLE FORM DSA 103

NOTE: THE EXAMPLE FORM DSA-103 SHOWN ON THIS SHEET IS FOR ILLUSTRATION PURPOSES ONLY TO ASSIST IN THE COMPLETION OF FUTURE PROJECT SPECIFIC FORM DSA-103'S. A FORM DSA-103 IS TO BE COMPLETED FOR EACH APPLICATION THAT THIS PC IS BEING INCORPORATED INTO AND THE EXAMPLE FORM DSA-103 IS TO BE CROSSED OUT ON THIS DRAWING.

Note: References are to the 2013 edition of the California Building Code (CBC) unless otherwise noted.

REQUIRED

TEST OR SPECIAL INSPECTION

TYPE

PERFORMED BY

-

SOILS

1. GENERAL: Table 1705A.6

X

a. Verify that:

- site has been prepared properly prior to placement of controlled fill and/or excavations for foundations.
- foundation excavations are extended to proper depth and have reached proper material, and
- materials below footings are adequate to achieve the design bearing capacity.

Periodic

GE\*

\* By geotechnical engineer or his or her qualified representative. Use PI in lieu of GE if there is no GE for the site specific project.

-

2. COMPACTED FILLS: Table 1705A.6

X

a. Perform qualification testing of fill materials.

Test

Lab\*

\* Under the supervision of the geotechnical engineer.

X

b. Verify use of proper materials and inspect lift thicknesses, placement, and compaction during placement of fill.

Continuous

GE\*

\* By geotechnical engineer or his or her qualified representative. Use PI in lieu of GE if there is no GE for the site specific project.

X

c. Test compaction of fill.

Test

Lab\*

\* Under the supervision of the geotechnical engineer.

-

4. CAST-IN-PLACE DEEP FOUNDATIONS (PIERS): Table 1705A.7

X

a. Inspect drilling operations and maintain complete and accurate records for each pier.

Continuous

PI

X

b. Verify locations of piers.

Continuous

PI

X

c. Confirm pier diameters, plumbness, bell diameters (if applicable), lengths, and embedment into bedrock (if applicable). Record concrete or grout volumes.

Continuous

GE\*

\* By geotechnical engineer or his or her qualified representative. Use PI in lieu of GE if there is no GE for the site specific project.

X

d. Confirm adequate end strata bearing capacity.

Test

Lab\*

\* Under the supervision of the geotechnical engineer.

X

e. Concrete piers.

Provide tests and inspections per CONCRETE section below.

-

CONCRETE Table 1705A.3

-

7. CAST IN PLACE CONCRETE

Material Verification and Testing:

X

a. Verify use of required design mix.

Periodic

SI & PI\*

\* To be performed by batch-plant special inspector and project inspector.

X

c. Perform slump, temperature, and (where required) air content tests.

Test

Lab

ASTM C172, ASTM C31.

X

d. Test concrete (compression).

Test

Lab

ACI 318 Section 5.6 and 1905A.1.2 (1913.3.1\*). ASTM C39.

-

Inspection:

X

e. Batch plant inspection

Continuous

SI

1705A.3.2. If approved by DSA, batch plant inspection may be reduced to periodic if plant complies with 1705A.3.3, Item 1, and requires first batch inspection, weighmaster, and batch tickets.

X

g. Inspect placement of formwork, reinforcing steel, embedded items and concrete. Inspect curing and form removal.

Continuous

PI\*

\* May be performed by a special inspector when specifically approved by DSA.

+

MASONRY TMS 402-11/ACI 530-11/ASCE 5-11 Table 1.19.3

-

STEEL Table 1705A.2.1

-

17. STRUCTURAL STEEL AND COLD-FORMED STEEL USED FOR STRUCTURAL PURPOSES

Material Verification:

X

a. Verify that all materials are appropriately marked and that:

- Mill certificates indicate material properties that comply with requirements.
- Material sizes, types and grades comply with requirements.

Periodic

-

\* By special inspector when performed off-site; by project inspector for steel shipped directly to project site without welding or fabrication.

X

b. Test unidentified materials

Test

Lab

2203A.1 (2203.1\*). ASTM A370.

X

c. Examine seam welds of structural tubes and pipes

Periodic

SI\*

\* DSA IR 17-3.

-

Inspection:

X

d. Verify member locations, bracing and all details constructed in the field.

Continuous

PI

X

e. Verify stiffener locations, connection tab locations and all construction details fabricated in the shop.

Periodic

SI

-

18. HIGH STRENGTH BOLTS: Material Verification of High-Strength Bolts, Nuts, and Washers:

X

a. Verify identification markings and manufacturer's certificates of compliance conform to ASTM standards specified in the DSA approved documents.

Periodic

SI

DSA IR 17-9

X

b. Test high-strength bolts, nuts and washers.

Test

Lab

2213A.1 (2212.6.1\*). ASTM F806, A370. DSA IR 17-8

-

Inspection of High-Strength Bolt Installation:

X

c. Bearing-type ("snug tight") connections.

Periodic

SI\*

DSA IR 17-9

-

19. WELDING: Verification of Materials, Equipment, Welders, etc:

X

a. Verify weld filler material identification markings per AWS designation listed on the DSA approved documents and the WPS.

Periodic

SI

X

b. Verify weld filler material manufacturer's certificate of compliance.

Periodic

SI

X

c. Verify WPS, welder qualifications and equipment.

Periodic

SI

DSA IR 17-3.

-

19.1 SHOP WELDING:

X

a. Inspect groove, multi-pass, and fillet welds > 5/16"

Continuous

SI

Per AISC 360 (and AISC 341 as applicable). DSA IR 17-3.

X

b. Inspect single-pass fillet welds ≤ 5/16"

Periodic

SI

Per AISC 360 (and AISC 341 as applicable). DSA IR 17-3.

+

WOOD

+

OTHER

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2013 CBC PC STRUCTURAL DESIGN NOTES

DESCRIPTION

DESIGN VALUES

DEAD AND LIVE LOADS

ROOF LIVE LOAD (Lr)

20 PSF

ROOF DEAD LOAD (D)

5 PSF

ALLOWABLE SOIL PRESSURE

DL

1000 PSF

DL+Lr

1000 PSF

DL+SNOW

1000 PSF

ROOF SNOW LOAD

GROUND SNOW LOAD (Pg)

22 PSF

SLOPED ROOF SNOW LOAD (Ps)

20 PSF

SNOW EXPOSURE FACTOR (Ce)

1.1

SNOW IMPORTANCE FACTOR (I)

1.0

THERMAL FACTOR (Ct)

1.2

FLOOD DESIGN

FLOOD HAZARD AREA

NO

WIND DESIGN

ULTIMATE DESIGN WIND SPEED (Vult)

130 MPH

WIND EXPOSURE FACTOR

C

TOPOGRAPHIC FACTOR (Kzt)

1.0

ASCE 7-10 WIND ANALYSIS METHOD

CHAPTER 27 DIRECTIONAL PROCEDURE

VELOCITY PRESSURE EXPOSURE COEFFICIENT (Kz)

0.85

NET PRESSURE COEFFICIENT

VARIES, SEE CALCULATIONS

WIND DIRECTIONALITY FACTOR (Kd)

0.85

WIND VELOCITY PRESSURE (qh)

31.3 PSF

SEISMIC DESIGN

LATERAL FORCE RESISTING SYSTEM

STEEL ORDINARY MOMENT RESISTING FRAMES

ASCE 7-10 ANALYSIS PROCEDURE

SECTION 12.8 EQUIVALENT LATERAL FORCE PROCEDURE

SEISMIC DESIGN CATEGORY

E

SEISMIC IMPORTANCE FACTOR

1.0

DESIGN BASE SHEAR (V)

1305 #

SEISMIC RESPONSE COEFFICIENT (Cs)

0.29

RESPONSE MODIFICATION FACTOR (R)

3.5

SYSTEM OVERSTRENGTH FACTOR (Do)

3.0

DEFLECTION AMPLIFICATION FACTOR (Cd)

3.0

SITE CLASS

D

MAPPED MCE, 5% DAMPED, SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD (Ss)

1.875

SHORT PERIOD SITE COEFFICIENT (F<sub>s</sub>)

1.0

DESIGN MCE, 5% DAMPED, SPECTRAL RESPONSE ACCELERATION AT SHORT PERIOD (S<sub>ds</sub>)

1.0

MAPPED MCE, 5% DAMPED, SPECTRAL RESPONSE ACCELERATION AT 1 SECOND PERIOD (S<sub>1</sub>)

1.3

LONG PERIOD SITE COEFFICIENT (F<sub>v</sub>)

1.5

DESIGN, 5% DAMPED, SPECTRAL RESPONSE ACCELERATION AT 1 SECOND PERIOD (S<sub>ds</sub>)

1.3

HORIZONTAL OR VERTICAL IRREGULARITY TYPES

NONE

BUILDING DATA

CONSTRUCTION CLASSIFICATION

TYPE II-B

OCCUPANCY CLASSIFICATION

A-2

RISK CATEGORY

II

NUMBER OF STORIES

1

MINIMUM SEISMIC SEPARATION

3"

BUILDING AREA

900 SF

NOTICE OF DISCLAIMER FOR STRUCTURAL ENGINEERING RESPONSIBILITY

1. PER TITLE 24, PART 1, SECTION 4-316(e) OF THE CALIFORNIA CODE OF REGULATIONS, THIS NOTICE SHALL BE GIVEN TO DSA PRIOR TO THE APPROVAL OF PLANS AND SPECIFICATIONS.

2. FOR THE SITE SPECIFIC PROJECT, ROGER HYTTINEN IS NOT THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE, UNLESS OTHERWISE NOTED.

3. FOR THE SITE SPECIFIC PROJECT, ROGER HYTTINEN'S RESPONSIBILITY IS LIMITED TO THE PREPARATION OF PLANS AND SPECIFICATIONS FOR THE SHELTER(S) OF THIS PC ONLY.

4. STRUCTURAL OBSERVATION OF CONSTRUCTION IS SPECIFICALLY EXCLUDED FROM ROGER HYTTINEN'S RESPONSIBILITY FOR THE SITE SPECIFIC PROJECT.

5. ALL CONSTRUCTION ACTIVITIES RELATED TO STRUCTURAL ENGINEERING SHALL BE DELEGATED TO A QUALIFIED ENGINEER BY THE DESIGN PROFESSIONAL IN GENERAL RESPONSIBLE CHARGE. THESE ACTIVITIES INCLUDE, BUT ARE NOT LIMITED TO, APPROVAL OF INSPECTOR QUALIFICATIONS, STRUCTURAL OBSERVATION OF CONSTRUCTION, REVIEW OF INSPECTION REPORTS, AND SIGNING OFF THE VERIFIED REPORT FOR COMPLETED WORK.

6. ROGER HYTTINEN WILL BE RESPONSIBLE FOR RESPONDING TO QUESTIONS PERTAINING TO THE PLANS AND SPECIFICATIONS FOR THE SHELTER(S) OF THIS PC WHICH ARISE DURING PLAN CHECK AND CONSTRUCTION.

7. IN THE EVENT THAT ROGER HYTTINEN IS REQUIRED TO PROVIDE STRUCTURAL OBSERVATION OF CONSTRUCTION, HE SHALL BE NOTIFIED IN WRITING PRIOR TO THIS REQUIREMENT BEING MADE. ALSO, HIS ANTICIPATED ADDITIONAL FEES FOR THIS ADDITIONAL WORK SHALL BE PAID IN ADVANCE, PRIOR TO ANY STRUCTURAL OBSERVATION OR CONSTRUCTION SERVICES BEING PERFORMED.

GENERAL NOTES

I. SHELTER DESIGN

A. THE STRUCTURAL DESIGN OF THE COMPONENTS AND CONNECTIONS OF THIS SHELTER ARE SUFFICIENT FOR EAVE HEIGHTS RANGING FROM 7' UP TO 12' TALL.

B. REQUIRED EAVE HEIGHT FOR EACH SITE SHALL BE DETERMINED BY OWNER.

C. THIS SHELTER HAS BEEN DESIGNED AS AN OPEN STRUCTURE. THE ADDITION OF ANY ENCLOSURE DIRECTLY ATTACHED TO THE SHELTER, SUCH AS WALLS, INSECT MESH, OR SHADE SCREENS, SHALL BE PROHIBITED AS INCREASED WIND FORCES MAY RESULT.

II. DESIGN AND CONSTRUCTION STANDARDS

A. THE DESIGN OF THIS STRUCTURE IS IN CONFORMANCE WITH THE FOLLOWING STANDARDS AND ALL PHASES OF CONSTRUCTION SHALL BE PERFORMED IN ACCORDANCE WITH THE FOLLOWING STANDARDS.

1. 2013 CALIFORNIA ADMINISTRATIVE CODE (CAC) (PART 1, TITLE 24, CCR)

2. 2013 CALIFORNIA BUILDING CODE (CBC), VOLUMES 1 AND 2 (PART 2, TITLE 24, CCR) (2012 INTERNATIONAL BUILDING CODE WITH 2013 CALIFORNIA AMENDMENTS)

3. 2013 CALIFORNIA ELECTRICAL CODE (PART 3, TITLE 24, CCR) (2011 NATIONAL ELECTRICAL CODE WITH 2013 CALIFORNIA AMENDMENTS)

4. 2013 CALIFORNIA MECHANICAL CODE (CMC) (PART 4, TITLE 24, CCR) (2012 UNIFORM MECHANICAL CODE WITH 2013 CALIFORNIA AMENDMENTS)

5. 2013 CALIFORNIA PLUMBING CODE (CPC) (PART 5, TITLE 24, CCR) (2012 UNIFORM PLUMBING CODE WITH 2013 CALIFORNIA AMENDMENTS)

6. 2013 CALIFORNIA ENERGY CODE (PART 6, TITLE 24, CCR)

7. 2013 CALIFORNIA FIRE CODE (CFC) (PART 7, TITLE 24, CCR) (2012 INTERNATIONAL FIRE CODE WITH 2013 CALIFORNIA AMENDMENTS)

8. 2013 CALIFORNIA GREEN BUILDING STANDARDS CODE (PART 11, TITLE 24, CCR)

9. 2013 CALIFORNIA REFERENCED STANDARDS CODE (PART 12, TITLE 24, CCR)

10. NFPA 13 - 2013

11. NFPA 72 - 2013

III. CONSTRUCTION CHANGES

A. CHANGES TO THE APPROVED PLANS AND SPECIFICATIONS SHALL BE MADE BY AN ADDENDUM OR CONSTRUCTION CHANGE DOCUMENT APPROVED BY THE DIVISION OF THE STATE ARCHITECT AS REQUIRED BY PART 1, TITLE 24, C.C.R.

IV. FOUNDATION

A. THE FOUNDATION SHALL REST ON SOUND SOIL THAT IS FREE OF ORGANIC AND DELETERIOUS MATERIALS AND CAPABLE OF SUPPORTING 1000 PSF VERTICAL BEARING PRESSURE.

B. FOR LATERAL LOADING, THE FOUNDATION HAS BEEN DESIGNED TO THE MINIMUM LATERAL BEARING VALUE IN CBC, TABLE 1805A.2. THIS IS 100 PSF/FT LATERAL BEARING.

C. FOUNDATION DESIGN SHOWN IS BASED ON SOIL CONDITIONS GIVEN IN NOTES A AND B, ABOVE. OWNER SHALL VERIFY ACTUAL SOIL CONDITIONS AT EACH JOB SITE AND ANY REQUIRED ADJUSTMENTS TO THE FOOTING DESIGN SHALL BE DESIGNED BY OTHERS.

V. CONCRETE

A. COMPRESSION STRENGTH OF ALL REINFORCED CONCRETE SHALL NOT BE LESS THAN 4000 PSI AT 28 DAYS.

B. REINFORCING BARS SHALL BE DEFORMED BARS CONFORMING TO THE REQUIREMENTS OF MINIMUM ASTM A615 GRADE 40 FOR #4 AND SMALLER BARS AND GRADE 60 FOR BARS LARGER THAN #4.

C. MINIMUM CONCRETE CLEAR COVER FOR REINFORCING BARS SHALL BE 3".

D. A CONCRETE MIX DESIGN IN ACCORDANCE WITH CBC SECTION CHAPTER 19A SHALL BE PERFORMED AND STAMPED BY A CIVIL ENGINEER LICENSED IN THE STATE OF CALIFORNIA. THE CONCRETE MIX DESIGN SHALL BE SUBMITTED TO THE INSPECTOR FOR RECORD PRIOR TO CONSTRUCTION.

E. THE MIX DESIGN SHALL MEET THE CRITERIA HEREIN AND SHALL BE PROPER FOR LOCAL CONDITIONS INCLUDING, BUT NOT LIMITED TO, FREEZING AND THAWING EXPOSURE, CHEMICAL AND SALT EXPOSURE, AND SOIL CORROSIVITY WHERE SUCH PROBLEMS EXIST.

F. NON-SHRINK GROUT OR DRY PACK SHALL BE A PREMIXED, NONMETALLIC FORMULA WITH A MINIMUM COMPRESSIVE STRENGTH OF 7000 PSI AT 28 DAYS AND HAVING THE FOLLOWING CHARACTERISTICS: NO SHRINKAGE AFTER PLACEMENT OR EXPANSION AFTER SET (ASTM C1090), ONE DAY COMPRESSIVE STRENGTH OF AT LEAST 3000 PSI (ASTM C109) AND INITIAL SET TIME OF NOT LESS THAN 45 MINUTES (ASTM C161). PROVIDE "HI-FLOW GROUT" OR "DRY PACK GROUT" BY EUCLID, OR AN APPROVED EQUAL.

VI. STRUCTURAL STEEL

A. STEEL PLATE SHALL CONFORM TO THE REQUIREMENTS OF ASTM A36.

B. HOLLOW STRUCTURAL SECTIONS (HSS) SHALL CONFORM TO THE REQUIREMENTS OF ASTM A500, GRADE B.

C. ALL STRUCTURAL STEEL SHALL BE IDENTIFIED BY MILL CERTIFICATE.

D. HIGH STRENGTH BOLTS (HSB) SHALL BE GALVANIZED AND SHALL CONFORM TO THE REQUIREMENTS OF ASTM A325-N. HIGH STRENGTH BOLTS SHALL BE TIGHTENED TO A SNUG TIGHT CONDITION PLUS AN ADDITIONAL HALF TURN.

E. ALL HIGH STRENGTH BOLTS SHALL HAVE CERTIFICATION.

F. WELDING SHALL CONFORM TO THE REQUIREMENTS OF THE AMERICAN WELDING SOCIETY'S SPECIFICATION FOR THE MATERIAL BEING WELDED. ALL WELDING SHALL BE PERFORMED BY AWS CERTIFIED WELDERS.

G. WELD ELECTRODES SHALL BE E70XX AND SHALL CONFORM TO THE REQUIREMENTS OF AWS D1.8-8.3 FOR DEMAND CRITICAL WELDS.

H. ALL WELDING SHALL BE APPROVED BY AN AWS CERTIFIED INSPECTOR.

I. STEEL FRAMING SHALL BE COATED WITH ANTI-GRAFFITI POLYESTER TOGIC POWDER COAT FINISH MEETING AAMA 2604-02 SPECIFICATIONS.

J. SHOP DRAWINGS OF ALL STRUCTURAL STEEL SHALL BE SUBMITTED TO HYTTINEN ENGINEERING FOR APPROVAL PRIOR TO FABRICATION.

K. ALL BOLT HOLE DIAMETERS SHALL BE EQUAL TO THE BOLT DIAMETER PLUS 1/16" U.N.O. BOLT HOLES FOR ANCHOR BOLTS SHALL BE EQUAL TO THE BOLT DIAMETER PLUS 1/8".

L. ANCHOR BOLTS SHALL CONFORM TO ASTM F1554, GRADE 36 AND SHALL BE HOT DIP GALVANIZED.

VII. ALUMINUM

A. INTERLOCKING SEAM ALUMINUM ROOF DECK SHALL BE ROLL FORMED FROM ALUMINUM ALLOY 3004-H181 AND SHALL CONFORM TO THE DECK PROFILE SHOWN ON THE DRAWINGS.

B. ALUMINUM ROOF DECK SHALL BE COATED WITH HEAT REFLECTIVE BASF ULTRA-COOL COATING OR APPROVED EQUAL.

C. EXTRUDED ALUMINUM RIDGE CAP SHALL BE FABRICATED FROM ALUMINUM ALLOY 6105-T5 AND SHALL CONFORM TO THE REQUIREMENTS SHOWN ON THE DRAWINGS.

D. EXTRUDED ALUMINUM FASCIA SHALL BE FABRICATED FROM ALUMINUM ALLOY 6063-T5. EXTRUDED ALUMINUM GUTTER SHALL BE FABRICATED FROM ALUMINUM ALLOY 6105-T5. ALUMINUM COMPONENTS SHALL CONFORM TO THE REQUIREMENTS SHOWN ON THE DRAWINGS.

E. EXTRUDED ALUMINUM RIDGE CAP, GUTTER, AND FASCIA SHALL BE COATED WITH ANTI-GRAFFITI POLYESTER TOGIC POWDER COAT FINISH MEETING AAMA 2604-02 SPECIFICATIONS.

VIII. SCREWS

A. SCREWS SHALL BE HILTI Kwik-PRO SELF DRILLING SCREWS WITH BOND SEAL WASHERS PER ICC ESR-2196 OR APPROVED EQUAL.

B. SCREWS ATTACHING TO STEEL SHALL BE 12-24 HEX WASHER HEAD (HW) #5 POINT SCREWS. SCREWS ATTACHING TO ALUMINUM SHALL BE 8-18 HEX WASHER HEAD (HW) #2 POINT SCREWS.

C. ALL SCREWS SHALL BE STAINLESS STEEL OR COATED WITH ZINC.

D. THE MANUFACTURER SHALL PROVIDE A SCREW CERTIFICATION LETTER STATING THAT SCREWS PROVIDED MATCH THE SIZE AND TYPE SPECIFIED HEREIN. THE CERTIFICATION LETTER SHALL BE SUBMITTED TO THE INSPECTOR OF RECORD PRIOR TO INSTALLATION.

IX. SHOP FABRICATION AND FIELD ASSEMBLY

A. ALL STRUCTURAL STEEL AND ALUMINUM COMPONENTS SHALL BE SHOP FABRICATED SO THAT FIELD ASSEMBLY OF CONNECTIONS CAN BE PERFORMED USING ONLY BOLTING AND SCREW PLACEMENT.

X. SPECIAL INSPECTION

A. THE OWNER SHALL EMPLOY A SPECIAL INSPECTOR TO PERFORM INSPECTION OF THE CONSTRUCTION OF THIS PC IN ACCORDANCE WITH THE REQUIREMENTS OF CHAPTER 17A OF THE 2013 CALIFORNIA BUILDING CODE (PART 2, TITLE 24, C.C.R.) AND THE DIVISION OF THE STATE ARCHITECT.

XI. FIRE LIFE SAFETY

A. AN AUTOMATIC FIRE PROTECTION SYSTEM MAY BE REQUIRED FOR THIS BUILDING DEPENDING ON SITE SPECIFIC REQUIREMENTS. WHERE REQUIRED, THE AUTOMATIC FIRE PROTECTION SYSTEM SHALL BE DESIGNED BY OTHERS.

B. THE DESIGN OF THIS SHELTER IS CAPABLE OF SUPPORTING THE WEIGHT OF A FIRE SPRINKLER SYSTEM (1.5 PSF).

C. THE METAL ROOFING COMPLIES WITH FIRE CLASSIFICATION B. THIS SHELTER HAS NOT BEEN DESIGNED FOR PLACEMENT WITHIN ANY FIRE HAZARD SEVERITY ZONE.

SITE SPECIFIC OPTIONS

TO BE COMPLETED PRIOR TO PLAN CHECK SUBMITTAL.

QUANTITY OF SHELTERS OF THIS PC AT THIS SITE

SHELTER EAVE HEIGHT (7'-6" MIN, 12' MAX)

CONCRETE SLAB OR ASPHALT PAVING (BY OTHERS) OVER FOOTINGS? ☐ YES ☐ NO

ROOF DOWNSPOUTS? ☐ YES ☐ NO

ALUMINUM "V" PLUGS IN ROOF VOIDS FOR BIRD CONTROL? ☐ YES ☐ NO

SHEET INDEX

NT30.0 30' NAVAJO SHELTER DESIGN NOTES, EXAMPLE FORM DSA 103

NT30.1 30' NAVAJO SHELTER PLANS AND ELEVATIONS

NT30.2 30' NAVAJO SHELTER SECTIONS AND DETAILS

PRE-CHECK (PC) DOCUMENT

CODE: 2013 CBC

A separate project application for construction is required.

FILE NO.: PC-058

APPL. NO.: 02-113840

HYTTINEN ENGINEERING

5458 Longley Lane, Suite B

Reno, Nevada 89511

(775) 826-3019 PHONE

(775) 826-3076 FAX

REGISTERED PROFESSIONAL ENGINEER

NO. 52752

EX. 12-11-14

STATE OF CALIFORNIA

IDENTIFICATION STAMP

ON OF THE STATE ARCHITECT

02 113840

ACTH PLAN 10/23/14

DATE 10/23/14

PC

PROJECT:

30' NAVAJO SHELTERS

AMERICANA BUILDING PRODUCTS

#2 Industrial Dr. - Salem, IL 62881

(800)851-0865 www.americana.com

SITE ADDRESS:

SHEET TITLE:

DESIGN NOTES, EXAMPLE FORM DSA 103

DRAWN

NVGI

CHECKED

R.H.

DATE

10/21/14

SCALE

AS NOTED

JOB NO.

44-14

DRAWING NO.

NT30

SHEET

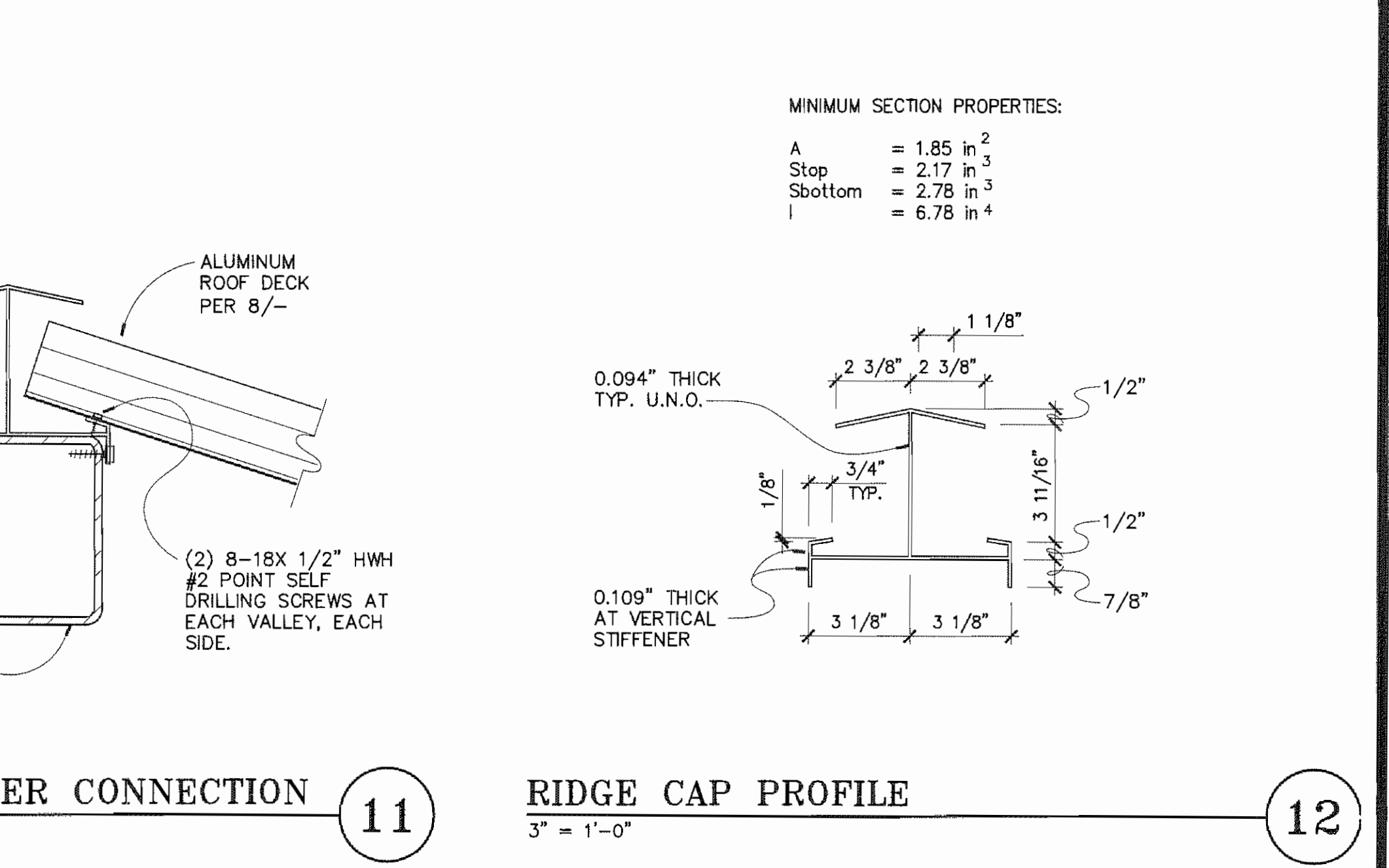
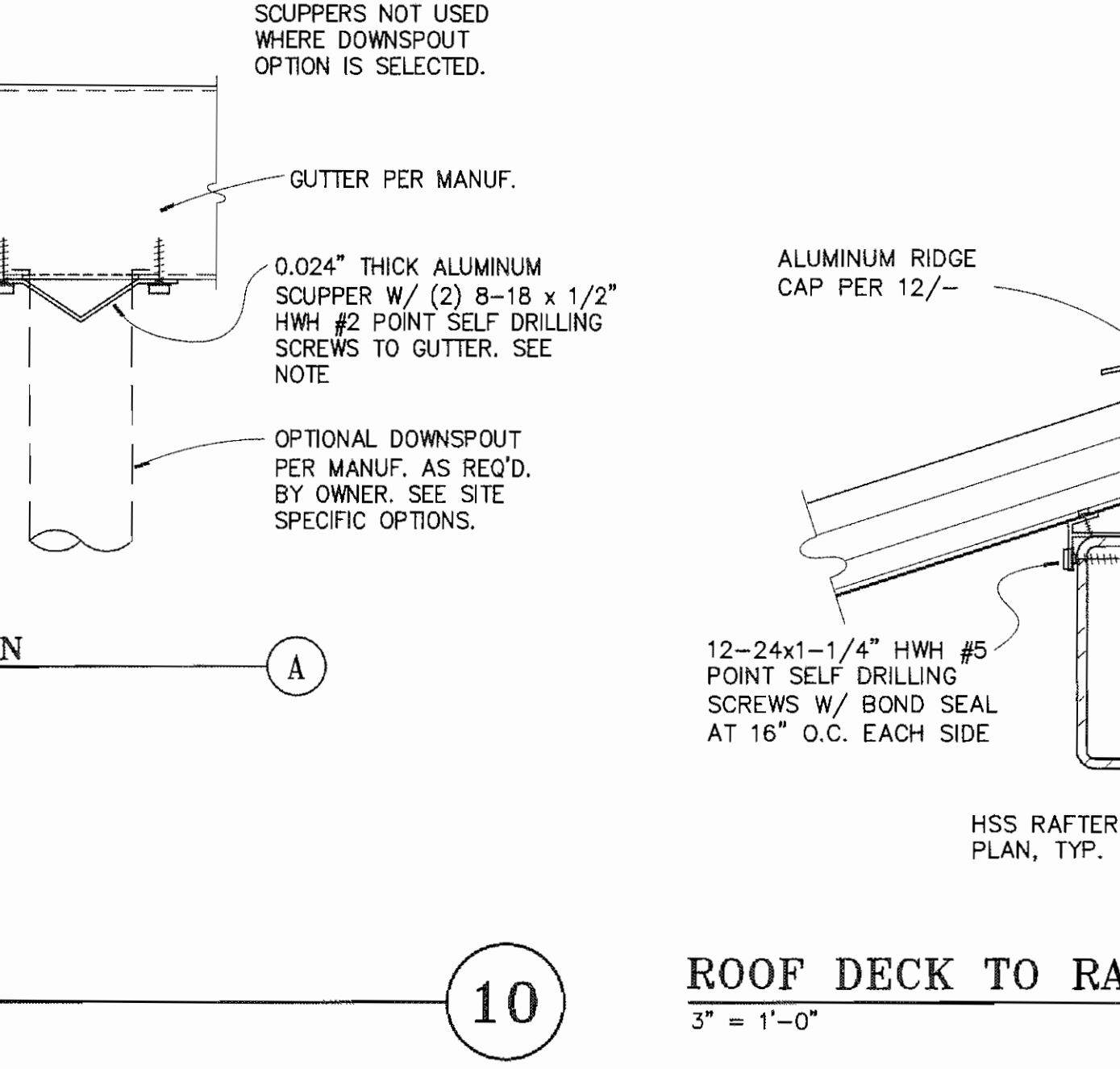
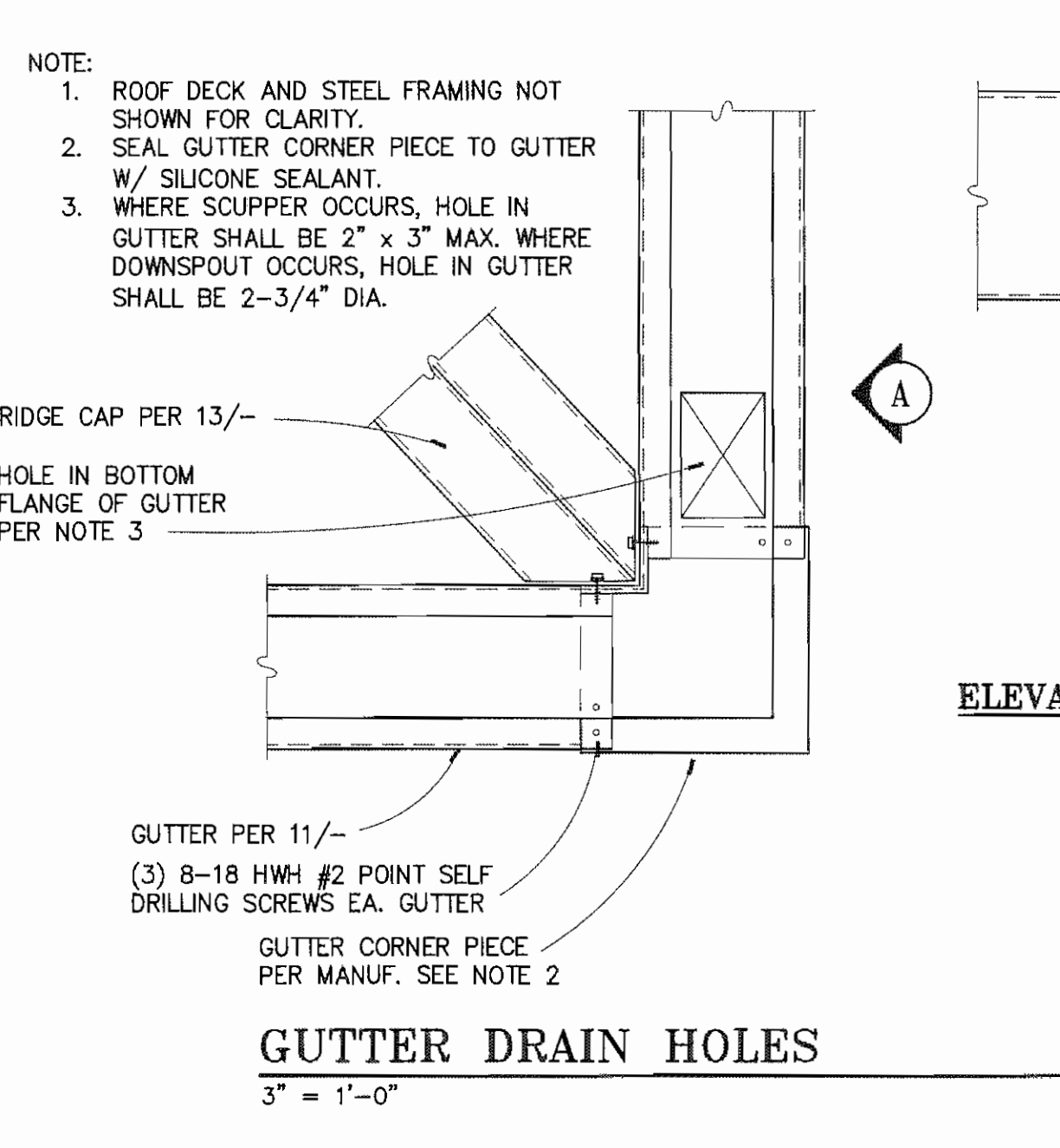
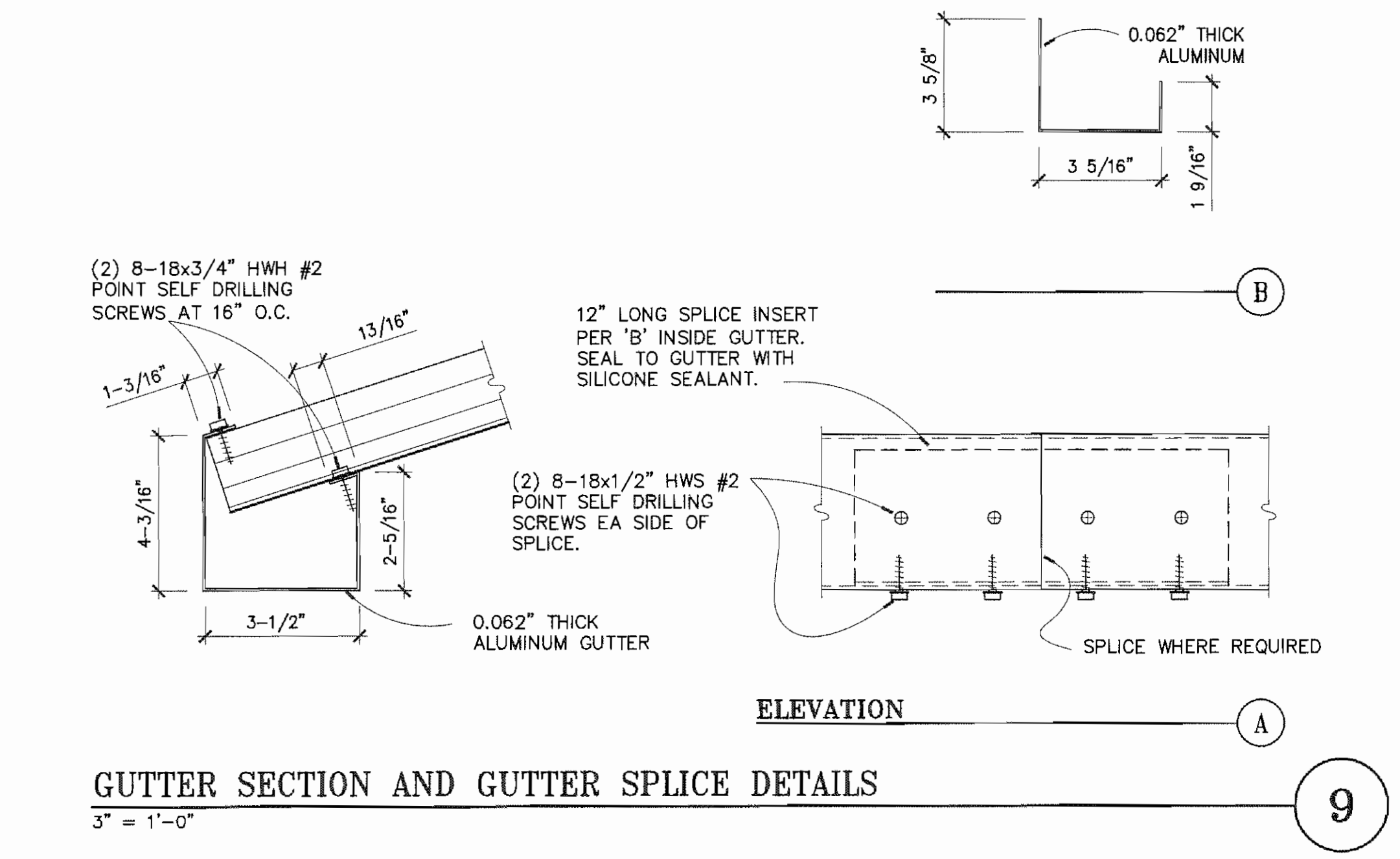
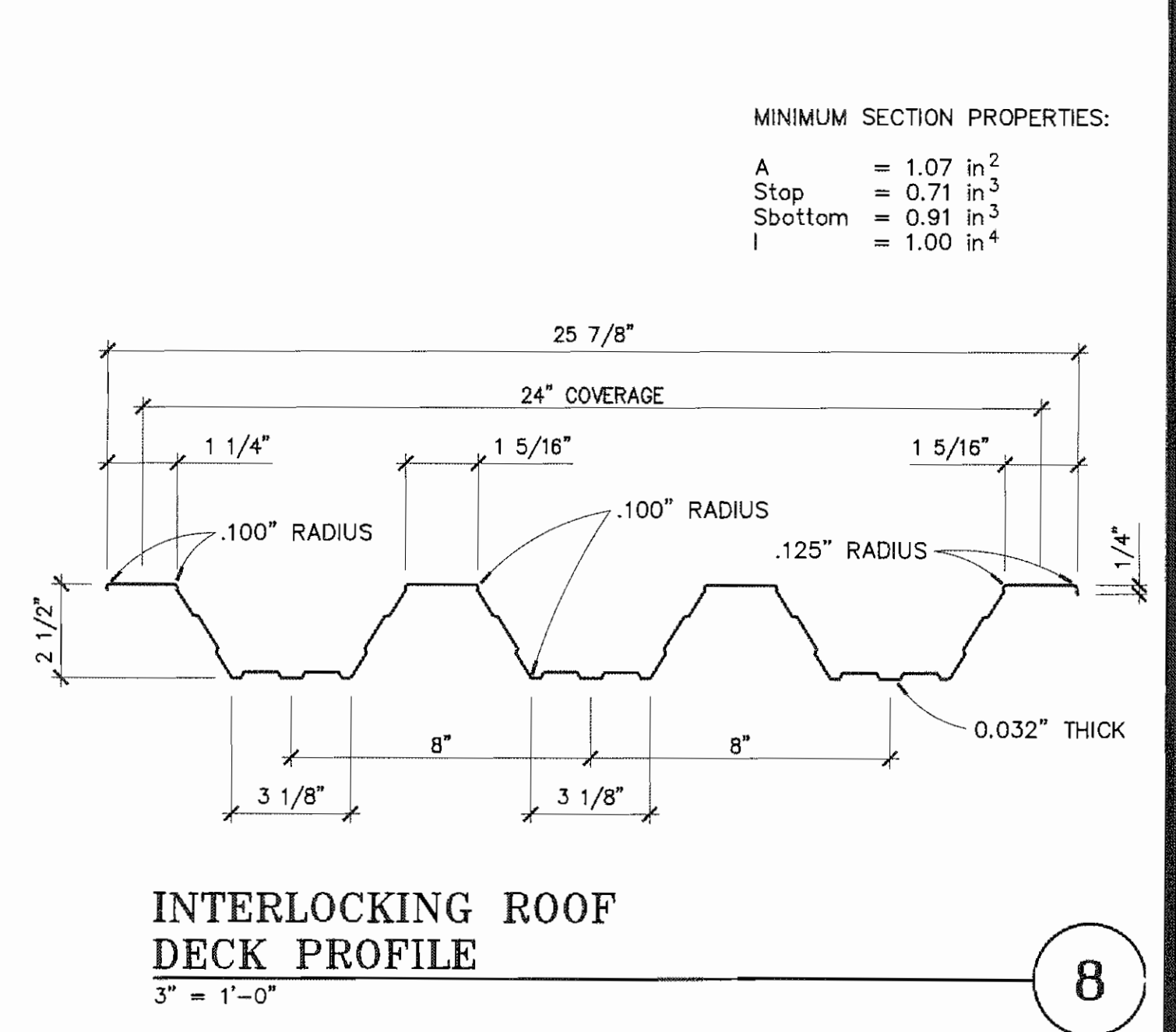
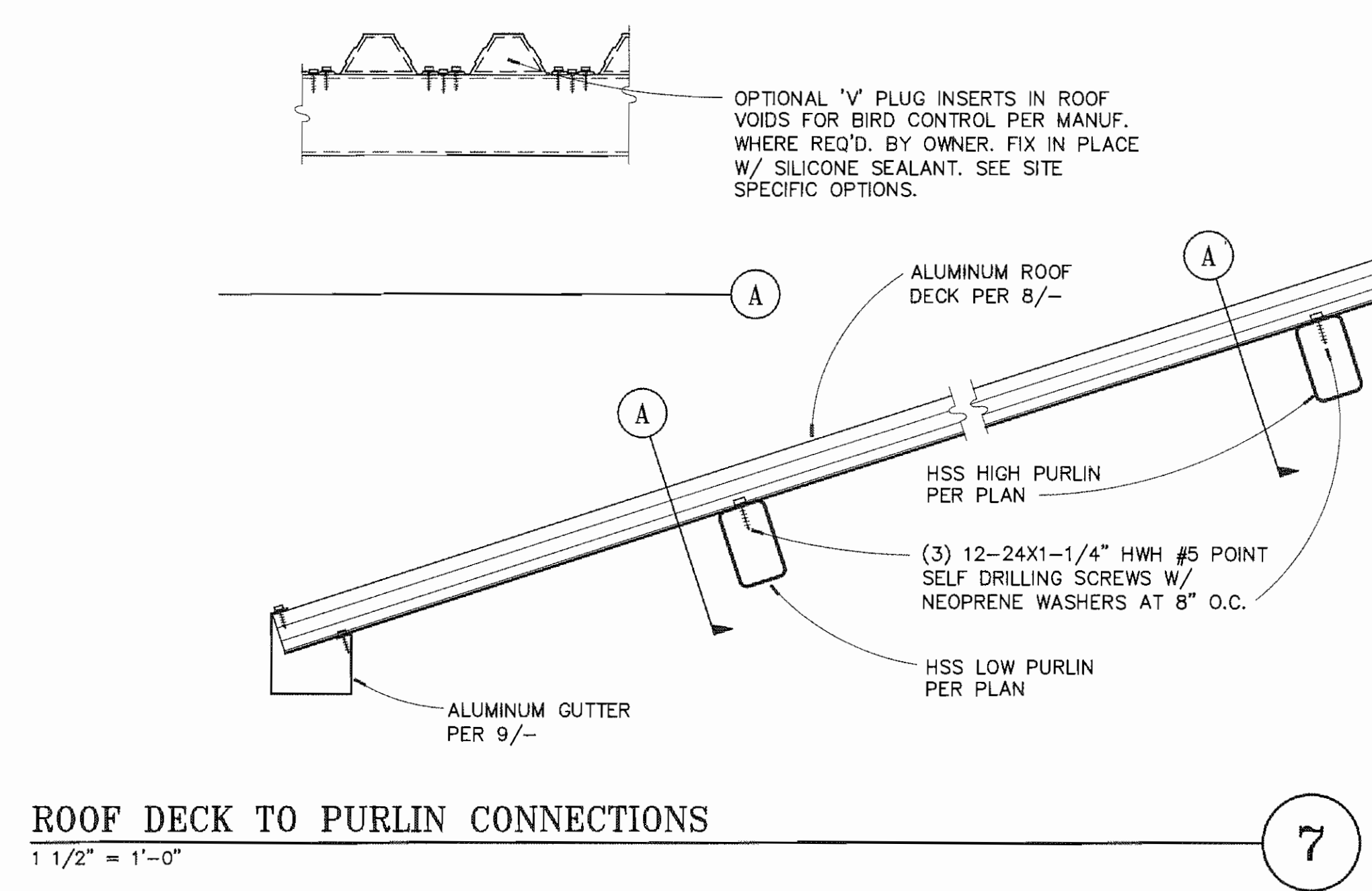
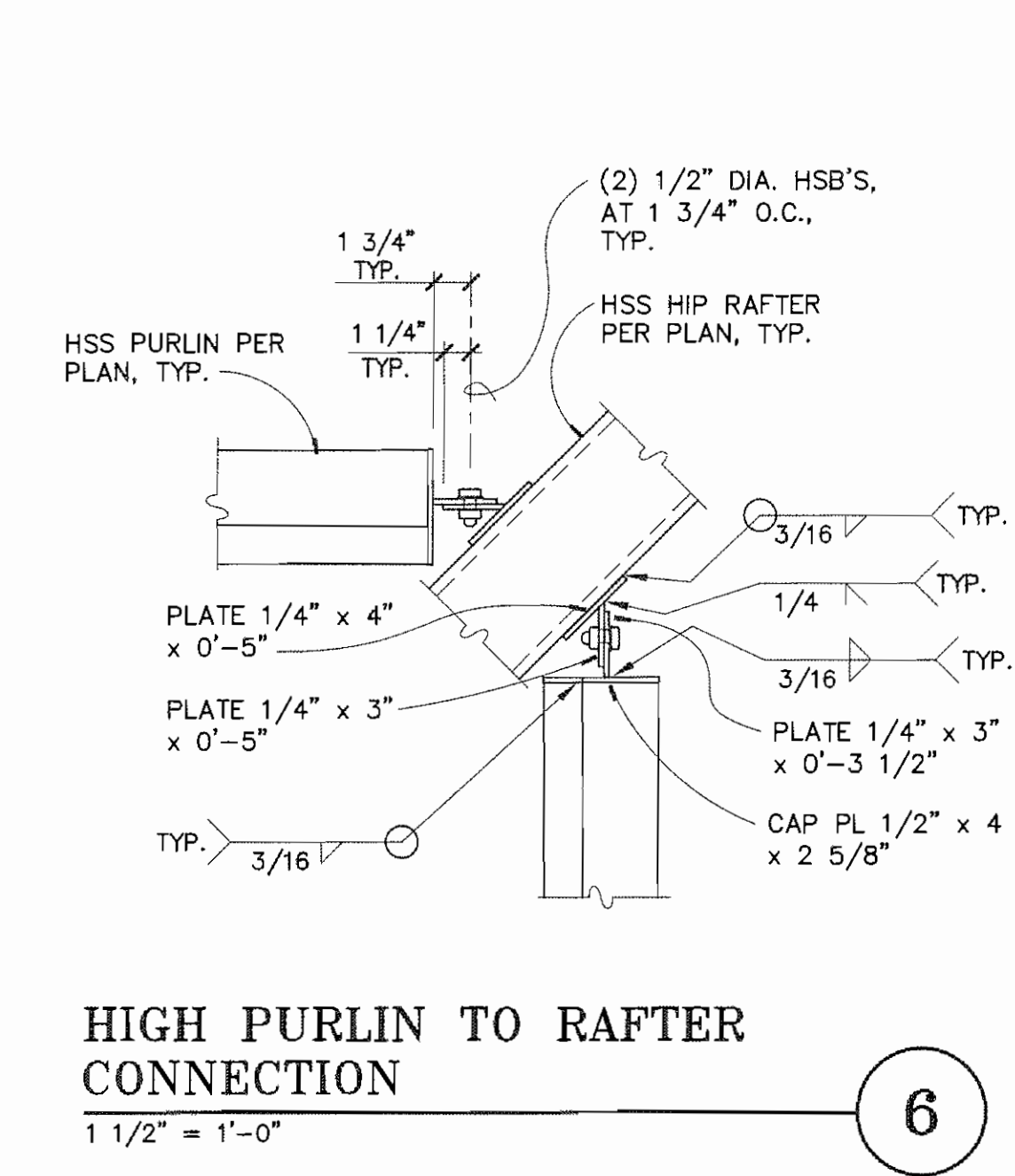
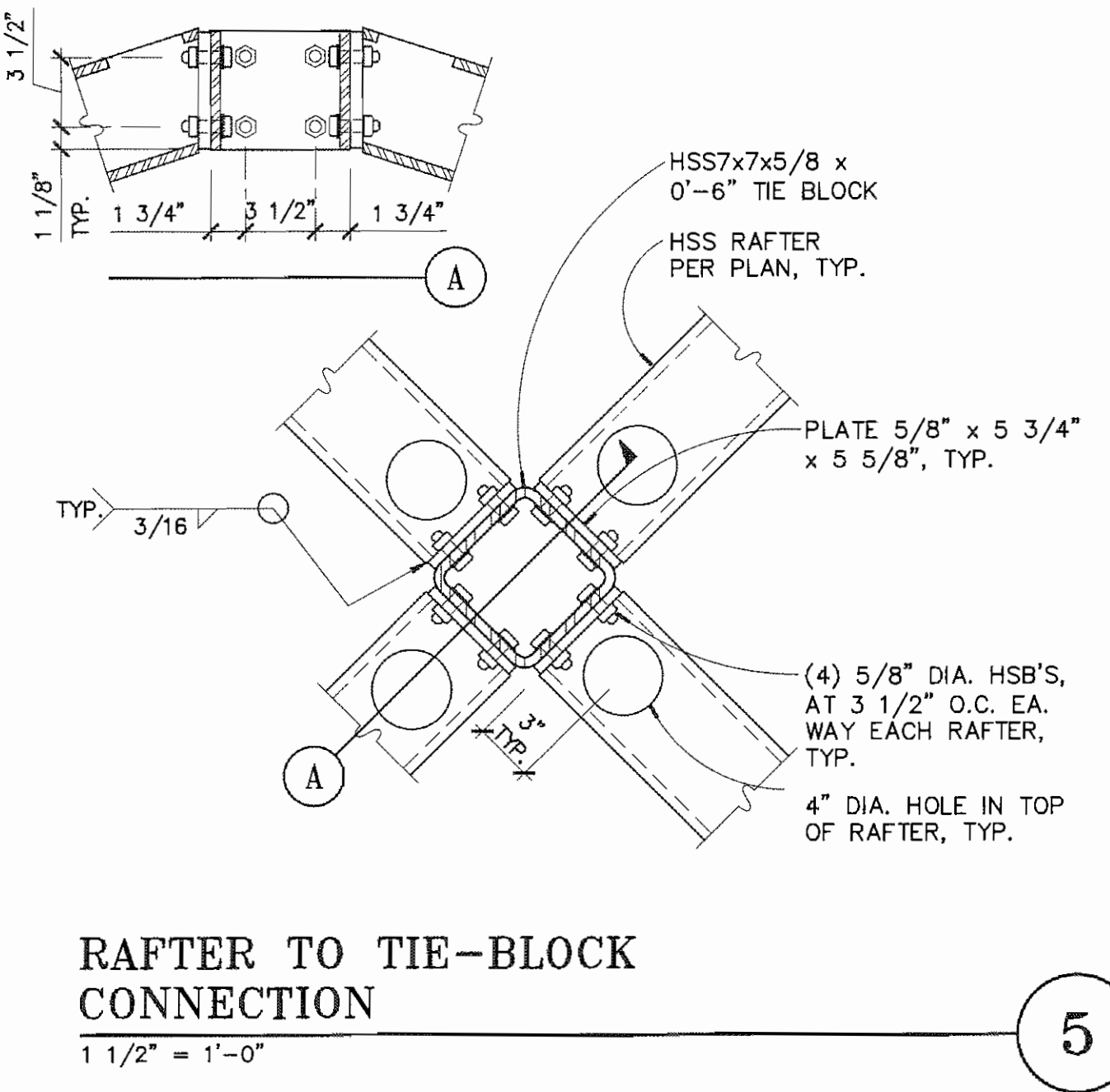
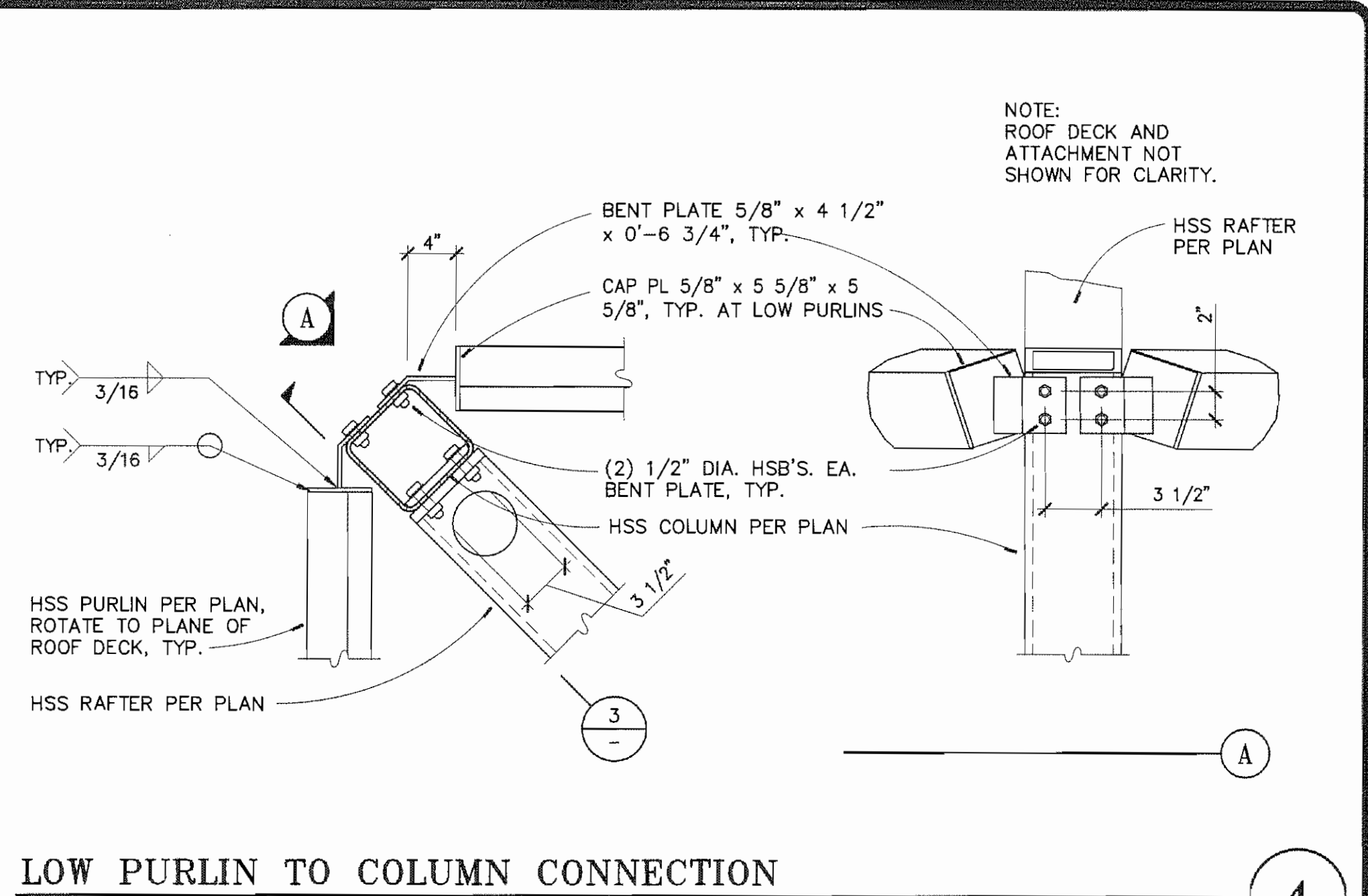
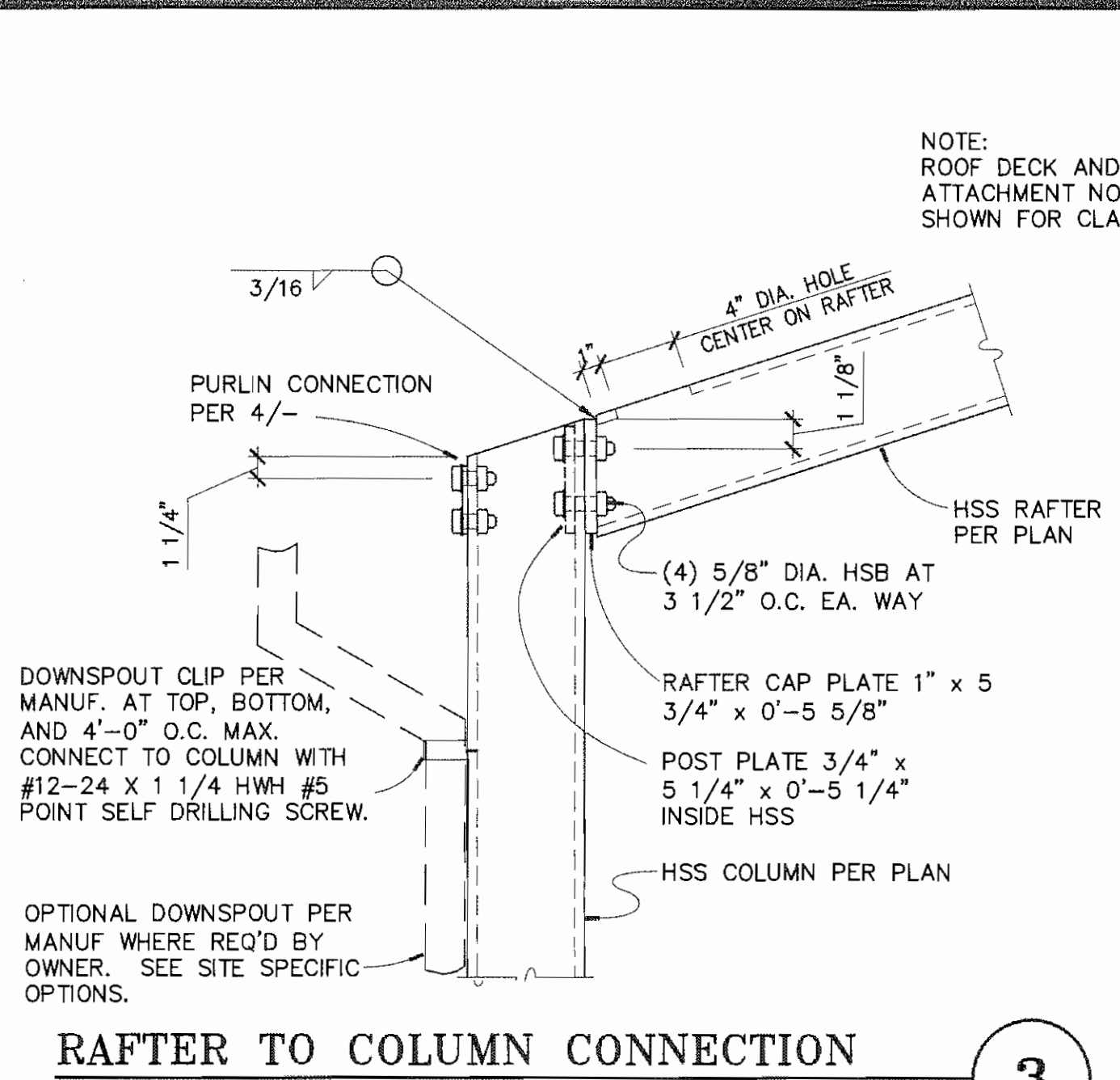
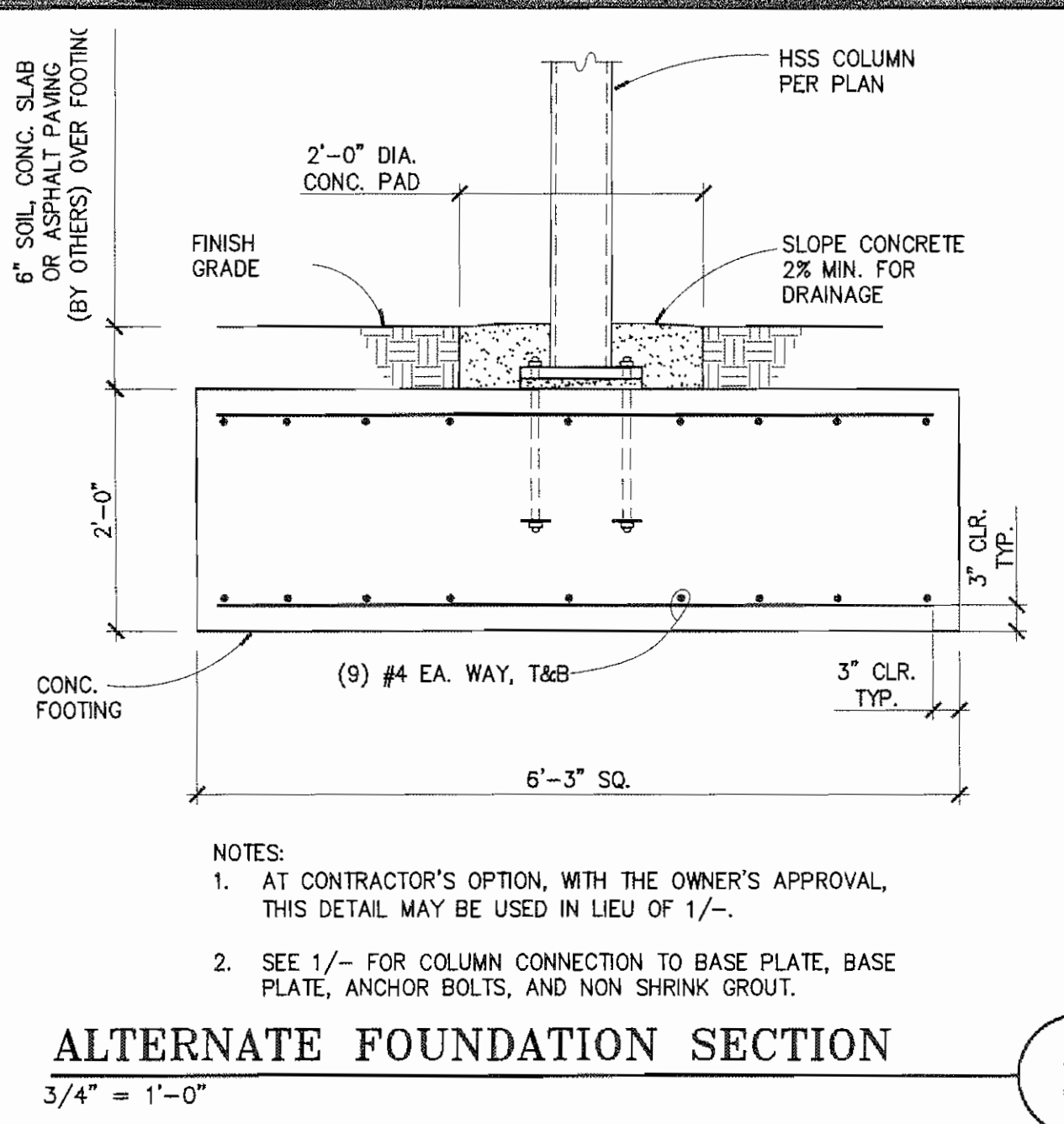
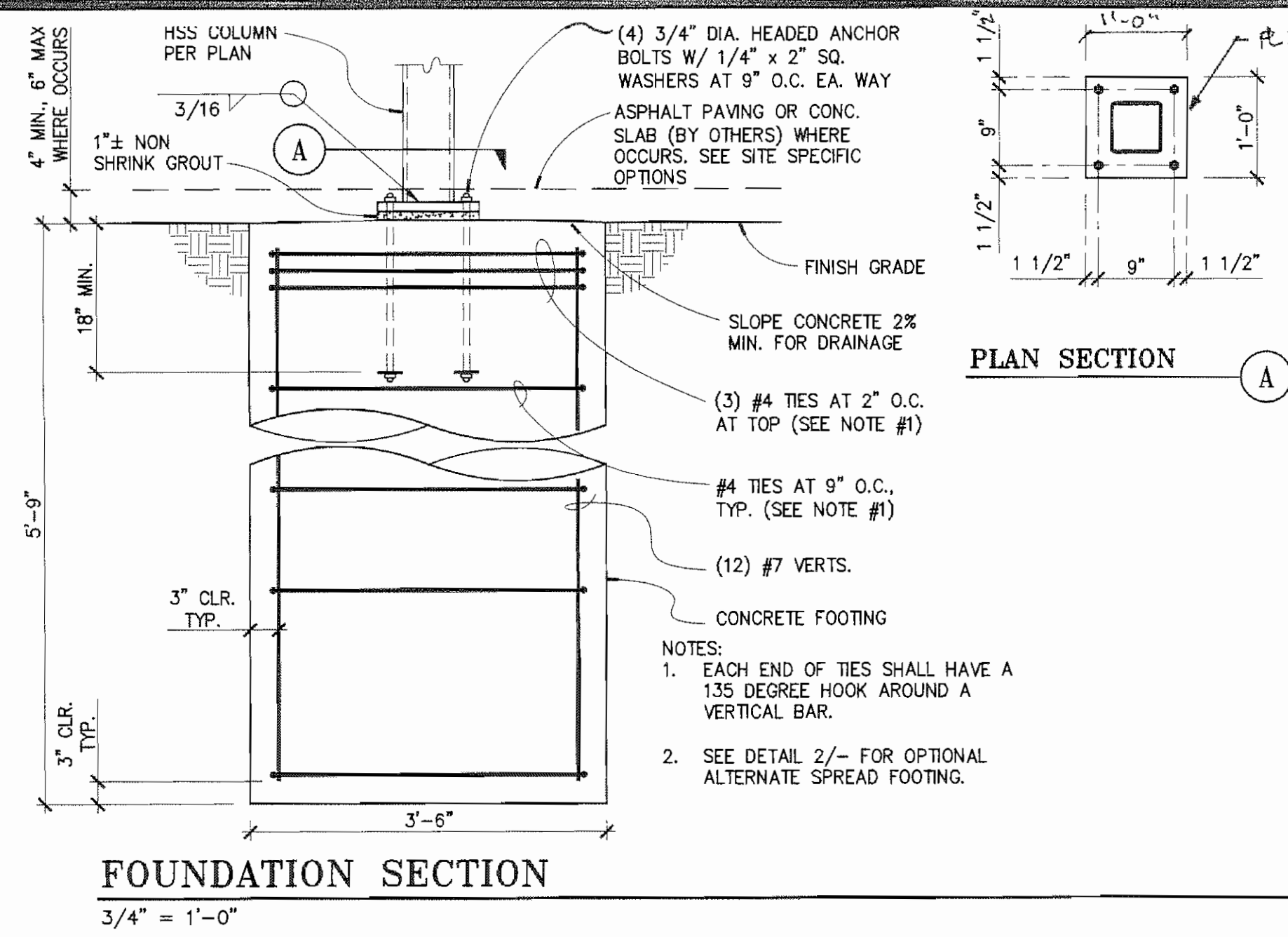
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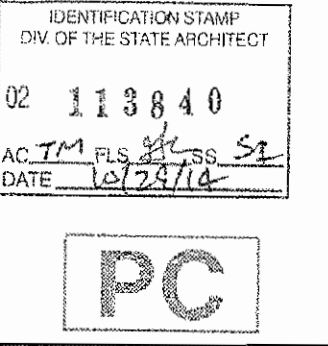
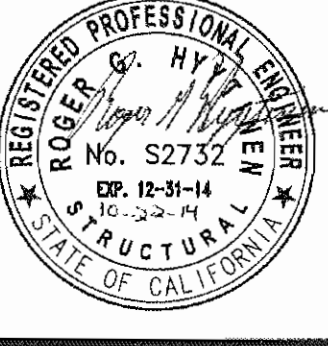




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